



DEPARTMENT OF HEALTH CARE ACCESS AND INFORMATION
FACILITIES DEVELOPMENT DIVISION

APPLICATION FOR HCAI PREAPPROVAL OF
MANUFACTURER'S CERTIFICATION (OPM)

OFFICE USE ONLY

APPLICATION #: OPM-0710

HCAI Preapproval of Manufacturer's Certification (OPM)

Type: New Renewal/Update

Manufacturer Information

Manufacturer: Omniceil, Inc

Manufacturer's Technical Representative: Jonathan Semchee

Mailing Address: 51 Pennwood Pl #400, Warrendale, PA 15086

Telephone: (724) 741-7706

Email: jonathan.semchee@omnicell.com

Product Information

Product Name: Omniceil IV dispenser units

OPM-0710

Product Type: Automatic IV Robotic Dispenser Unit

Product Model Number: IVX Station

BY: William Staehlin

General Description: Automated medication compounding

DATE: 02/25/2024

Applicant Information

Applicant Company Name: Omniceil, Inc

Contact Person: Jonathan Semchee

Mailing Address: 51 Pennwood Pl #400, Warrendale, PA 15086

Telephone: (724) 741-7706

Email: jonathan.semchee@omnicell.com

Title: Mechanical Engineer

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Registered Design Professional Preparing Engineering Recommendations

Company Name: DEGENKOLB ENGINEERS

Name: Chad Closs California License Number: S5946

Mailing Address: 225 Broadway, Suite 1325, San Diego, CA 92101

Telephone: (858) 699-5412 Email: ccloss@degenkolb.com

HCAI Special Seismic Certification Preapproval (OSP)

Special Seismic Certification is preapproved under OSP OSP Number: _____

Certification Method

Testing in accordance with: ICC-ES AC156 FM 1950-16

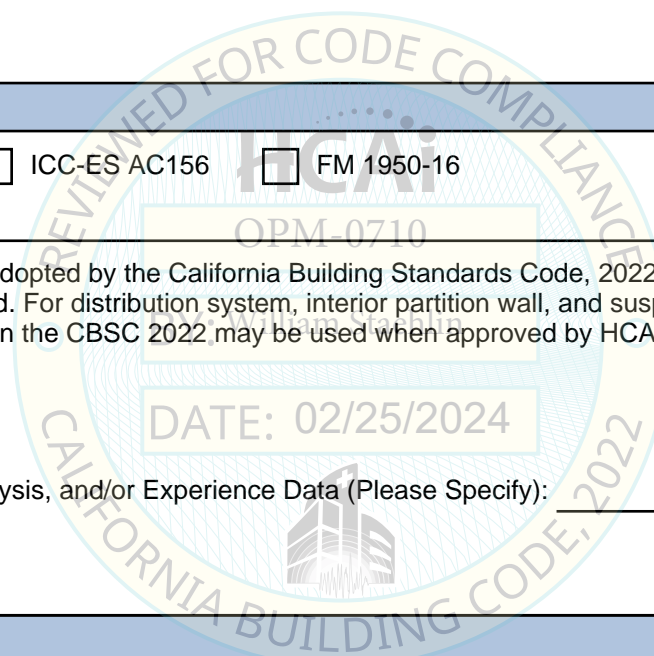
Other(s) (Please Specify): _____

*Use of criteria other than those adopted by the California Building Standards Code, 2022 (CBSC 2022) for component supports and attachments are not permitted. For distribution system, interior partition wall, and suspended ceiling seismic bracings, test criteria other than those adopted in the CBSC 2022 may be used when approved by HCAI prior to testing.

Analysis

Experience Data

Combination of Testing, Analysis, and/or Experience Data (Please Specify): _____



HCAI Approval

Date: 2/25/2024

Name: William Staehlin Title: Senior Structural Engineer

Condition of Approval (if applicable): _____

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OPM-0710

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OMNICELL IVX



MODELS
IVX STATION

GENERAL NOTES

1. THIS HCAI PRE-APPROVAL OF MANUFACTURER'S CERTIFICATION (OPM) IS BASED ON THE CBC 2022. THE DEMAND (DESIGN FORCES) FOR USE WITH THIS OPM SHALL BE BASED ON THE CBC 2022.

2. PRE-APPROVED DESIGN AND MATERIALS CONFORM WITH THE 2022 EDITION OF THE CALIFORNIA BUILDING CODE. DETAILS WITHIN THIS APPROVAL MAY BE USED ANYWHERE IN THE STATE OF CALIFORNIA WHERE $S_{DS} \leq 2.5$

3. SEISMIC FORCES ON EQUIPMENT DETERMINED PER THE 2022 CBC & ASCE 7-16. ALL LOADS BELOW ARE FACTORED LOADS THAT SHALL BE USED FOR STRENGTH DESIGN.

4. EQUIPMENT MAY BE MOUNTED TO AN ELEVATED SLAB AT ANY FLOOR USING THE THROUGH BOLT CONDITION OR TO A NORMAL WEIGHT CONCRETE SLAB ON GRADE. THE MINIMUM REQUIRED SLAB PROPERTIES AND FACTORS USED TO CALCULATE THE SEISMIC DEMANDS ARE AS FOLLOWS:

CASE 1 (EQUIPMENT ABOVE GRADE TO ROOF, THRU-BOLT OPTION):
(8 TOTAL) THRU-BOLT

CONCRETE ON METAL DECK
 $f'_c \geq 3000$ PSI
NORMAL OR SAND LIGHT-WEIGHT CONCRETE
SEE FIGURE ON PAGE 3 FOR MINIMUM STEEL DECK REQUIREMENTS

$S_{DS} \leq 2.50$, $I_p = 1.5$, $z/h \leq 1.0$
 $R_p = 6.0$, $a_p = 2.5$, $\Omega_o = 1.0$ (TENSION), $\Omega_o = 2.5$ (SHEAR)

i. $F_p = 1.88W_p$, $F_v = 0.5W_p$

CASE 2 (EQUIPMENT AT OR BELOW GRADE, EXPANSION ANCHOR OPTION):
(16 TOTAL) 3/8" HILTI KB-TZ2 EMBED LENGTH = 2"

4" \geq THICKNESS \leq 5.5"
 $f'_c \geq 3000$ PSI
NORMAL WEIGHT CONCRETE
PROVIDE 12" MIN DISTANCE TO OPENINGS OR THE EDGE OF SLAB

$S_{DS} \leq 2.50$, $I_p = 1.5$, $z/h = 0$
 $R_p = 6.0$, $a_p = 2.5$, $\Omega_o = 2.5$

i. $F_p = 1.88W_p$, $F_v = 0.5W_p$

CASE 3 (EQUIPMENT AT OR BELOW GRADE, EXPANSION ANCHOR OPTION):
(8 TOTAL) 5/8" HILTI KB-TZ2 EMBED LENGTH = 3.25"

THICKNESS ≥ 5.5 "
 $f'_c \geq 3000$ PSI
NORMAL WEIGHT CONCRETE
PROVIDE 12" MIN DISTANCE TO OPENINGS OR THE EDGE OF SLAB

$S_{DS} \leq 2.50$, $I_p = 1.5$, $z/h = 0.0$
 $R_p = 6.0$, $a_p = 2.5$, $\Omega_o = 2.5$

i. $F_p = 1.88W_p$, $F_v = 0.5W_p$

6. THE STRUCTURAL ENGINEER-OF-RECORD (S.E.O.R.) OR PRINCIPAL-IN-CHARGE OF A PROJECT SPECIFIC SITE IS RESPONSIBLE FOR THE FOLLOWING:

- a. VERIFY THAT THE ATTACHMENTS ARE A MINIMUM 12" FROM ANY OPENINGS OR EDGES.
- b. VERIFY THAT THE ATTACHMENTS ARE 12" MINIMUM DISTANCE FROM ANY NEW OR EXISTING ANCHORS.
- c. DESIGN ANY SUPPLEMENTARY MEMBERS TO WHICH THE UNIT IS ATTACHED, TO SUPPORT WEIGHTS AND FORCES SHOWN. VERIFY THE ADEQUACY OF ANY EXISTING MEMBERS AND THEIR ATTACHMENTS FOR THE FORCES EXERTED ON THEM BY THE UNIT IN ADDITION TO ALL OTHER LOADS AND FORCES.
- d. VERIFY THAT THE INSTALLATION IS IN CONFORMANCE WITH THE 2022 CBC AND WITH THE DETAILS SHOWN IN THIS PRE-APPROVAL. VERIFY THAT THE EQUIPMENT'S ACTUAL WEIGHT, CG LOCATION, ANCHOR LOCATIONS, DETAILS AND THE MATERIAL AND GAGE OF THE UNIT WHERE ATTACHMENTS ARE MADE AGREE WITH THE INFORMATION SHOWN IN THIS PRE-APPROVAL.
- e. THE ATTACHMENTS TO THE ELEVATED AND ON GRADE SLABS HAVE BEEN EVALUATED FOR THE WORST CASE LOADING PER THE 2022 CBC. STRUCTURAL ENGINEER-OF-RECORD (S.E.O.R.) OR PRINCIPAL-IN-CHARGE OF A SITE SPECIFIC PROJECT SHALL EVALUATE THE ATTACHMENT FOR CONDITIONS THAT VARY FROM THIS PRE-APPROVAL.

7. THIS OPM COVERS ONLY THE SUPPORTS AND ATTACHMENTS OF THE UNIT TO THE STRUCTURE.

8. EXPANSION OR WEDGE ANCHORS INTO CONCRETE: HILTI KB-TZ2 (ICC ESR-4266). INSTALL ANCHORS IN ACCORDANCE WITH THE ICC REPORT AND MANUFACTURER'S RECOMMENDATIONS. TEST AT LEAST 50% OF ANCHORS. TESTS SHALL BE CONDUCTED IN THE PRESENCE OF THE SPECIAL INSPECTOR AND A REPORT OF THE TEST RESULTS SHALL BE SUBMITTED TO HCAI.

TEST PER ONE OF THE FOLLOWING METHODS:

- a. DIRECT PULL TENSION TEST. ANCHOR IS ACCEPTABLE IF NO MOVEMENT IS OBSERVED FOR A MINIMUM OF 15 SECONDS AT THE TEST LOAD GIVEN IN TABLE ON THE FOLLOWING PAGE. MOVEMENT MAY BE DETERMINED WHEN THE WASHER UNDER THE NUT BECOMES LOOSE.
- b. TORQUE WRENCH TEST: TEST ANCHORS TO THE REQUIRED TORQUE LOAD GIVEN IN TABLE ON THE FOLLOWING PAGE WITHIN THE LIMIT OF ONE-HALF TURN OF THE NUT.

ANCHOR TEST LOAD VALUES						
ANCHOR DIAMETER (IN)	EMBED hef (IN)	TENSION LOAD (LBS)	TORQUE LOAD (FT-LB)	CONCRETE TYPE	MINIMUM EDGE DISTANCE	MINIMUM SPACING
3/8" SS	2"	6,180	30	NORMAL WEIGHT	12"	11"
5/8" SS	3-1/4"	18,835	60	NORMAL WEIGHT	12"	11"
3/8" SS	2"	SEE NOTE a	30	SAND LIGHT-WEIGHT	12"	11"
3/8" CS	1-1/2"	6,490	30	NORMAL WEIGHT	12"	11"

- c. TEST 3/8" EXPANSION ANCHORS USING THE TORQUE WRENCH TEST METHOD PER MANUFACTURER'S RECOMMENDATION AND AS DESCRIBED IN THIS PAGE.
- d. IF ANY ANCHOR FAILS TESTING, ALL ANCHORS OF THE SAME TYPE SHALL BE TESTED, WHICH ARE INSTALLED BY THE SAME TRADE, NOT PREVIOUSLY TESTED UNTIL TWENTY CONSECUTIVE ANCHORS PASS, THEN RESUME THE INITIAL TEST FREQUENCY PER CBC 2022 1910A.5.1.



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GENERAL NOTES

9. IF ANY ANCHOR FAILS DURING TESTING, UNIT MUST BE MOVED SO THAT NO ANCHOR IS WITHIN 11" OF AN ABANDONED ANCHOR.

10. CONTRACTOR OR SEOR MUST VERIFY ANCHOR SPACING TO ADJACENT EQUIPMENT ANCHORS IS TO BE GREATER THAN 12".

11. ALL MISCELLANEOUS STEEL SHALL CONFORM TO THE FOLLOWING, UNLESS OTHERWISE NOTED:

THROUGH BOLTS	A307 GR. A.
STEEL ANGLES	A36

12. THE TABLE ON PAGE 3 SHOWS THE MOST CRITICAL FORCES CALCULATED FOR THE SUPPORT AND ATTACHMENT DESIGN.

13. FOR THE SUPPORT AND ATTACHMENT DESIGN, THE MOST CRITICAL LOAD COMBINATION IS (0.9 - 0.2Sds) D + E.

14. WHEN $z/h = 0$, THE DESIGN FORCES FOR THE EXPANSION ANCHORS INTO CONCRETE WERE SCALED UP BY Ω_o AS REQUIRED BY ASCE 7-16, SUPPLEMENT NO. 1, TABLE 13.6-1.

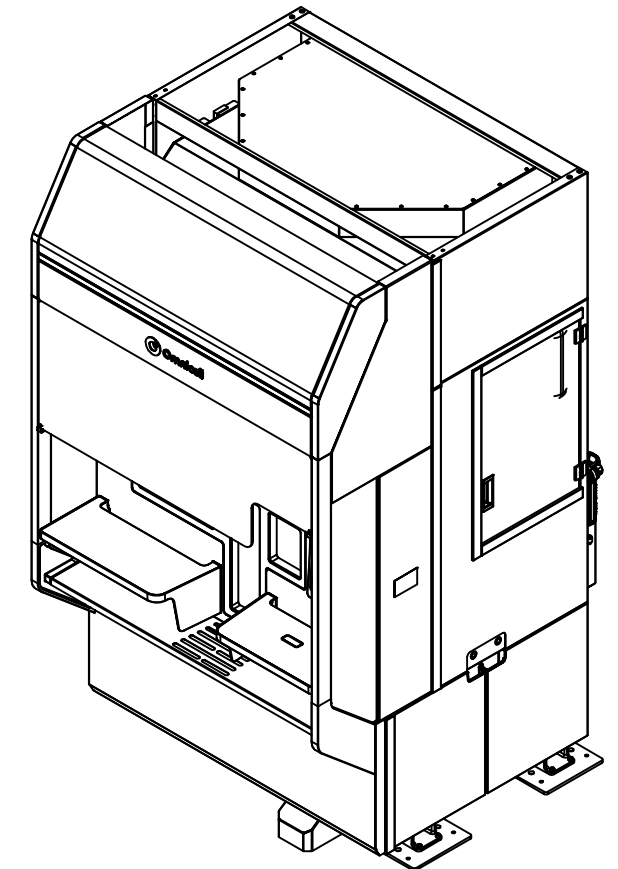
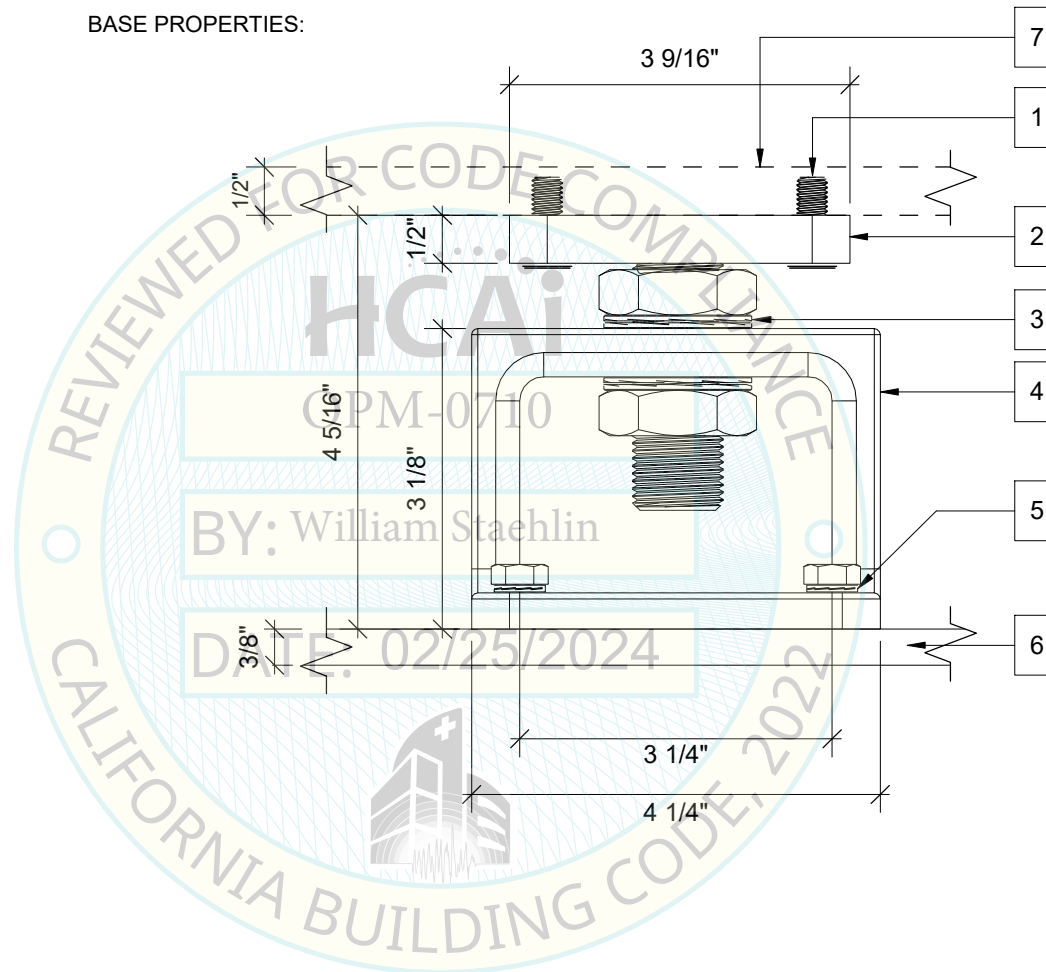
15. $T_{ult} + q$ IS THE FORCE DEMAND IN THE ANCHOR INCLUDING EFFECTS OF PRYING

16. CENTER OF GRAVITY (C.G.) WEIGHT IS A MAXIMUM. THIS PREAPPROVAL ENCOMPASSES ALL WEIGHTS UP TO THE MAXIMUM SHOWN.

17. EQUIPMENT MANUFACTURER MUST DESIGN UNIT TO MAKE C.G. EQUAL OR LESS THAN THE C.G. HEIGHT DIMENSION SHOWN ON ELEVATION ON PAGE 4-6.

18. WHEN INSTALLING DRILLED-IN ANCHORS IN EXISTING NON-PRESTRESSED REINFORCED CONCRETE, USE CARE AND CAUTION TO AVOID CUTTING OR DAMAGING THE EXISTING REINFORCING BARS. WHEN INSTALLING THEM INTO EXISTING PRESTRESSED CONCRETE (PRE- OR POST-TENSIONED) LOCATE THE PRESTRESSED TENDONS BY USING A NON-DESTRUCTIVE METHOD PRIOR TO INSTALLATION. EXERCISE EXTREME CARE AND CAUTION TO AVOID CUTTING OR DAMAGING THE TENDONS DURING INSTALLATION. MAINTAIN A MINIMUM CLEARANCE OF ONE INCH BETWEEN THE REINFORCEMENT AND THE DRILLED-IN ANCHOR.

BASE PROPERTIES:



ISOMETRIC VIEW

COMPONENT	MATERIAL	GRADE	STRENGTH [KSI]		MODULUS [KSI]		TORQUE LOAD (FT-LB)	
			YIELD	TENSILE	ELASTIC	SHEAR		
1	M8x1.25 SHCS	AISI 316	A4-50	30.5	72.5	28,300	10,900	11.5
2	FRAME MOUNTING PLATE	AISI 4340		68	108	27,800	11,700	
3	M24x3 THREADED ROD	AISI 4340	12.9	159	170	29,000	11,300	1000
4	BASEPLATE MOUNT	AISI 4340		68	108	27,800	11,700	
5	M8x1.25 HEX CAP	AISI 316	A4-70	65	100			11.5
6	BASEPLATE	ASTM A316L		25	70	28,000	11,500	
7	FRAME	AISI 304		30	75	27,500	10,900	



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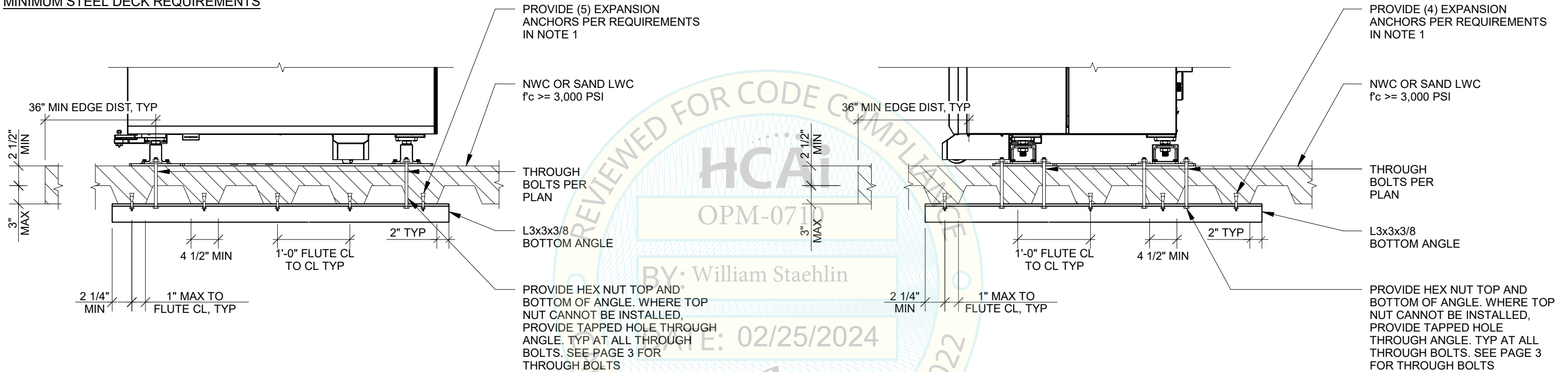
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MINIMUM STEEL DECK REQUIREMENTS



**THROUGH BOLT ASSEMBLY WHEN
METAL DECK RIBS ARE
PERPENDICULAR TO THE LENGTH OF
THE BASE OF THE EQUIPMENT**

**THROUGH BOLT ASSEMBLY WHEN
METAL DECK RIBS ARE PARALLEL TO
THE LENGTH OF THE BASE OF THE
EQUIPMENT**

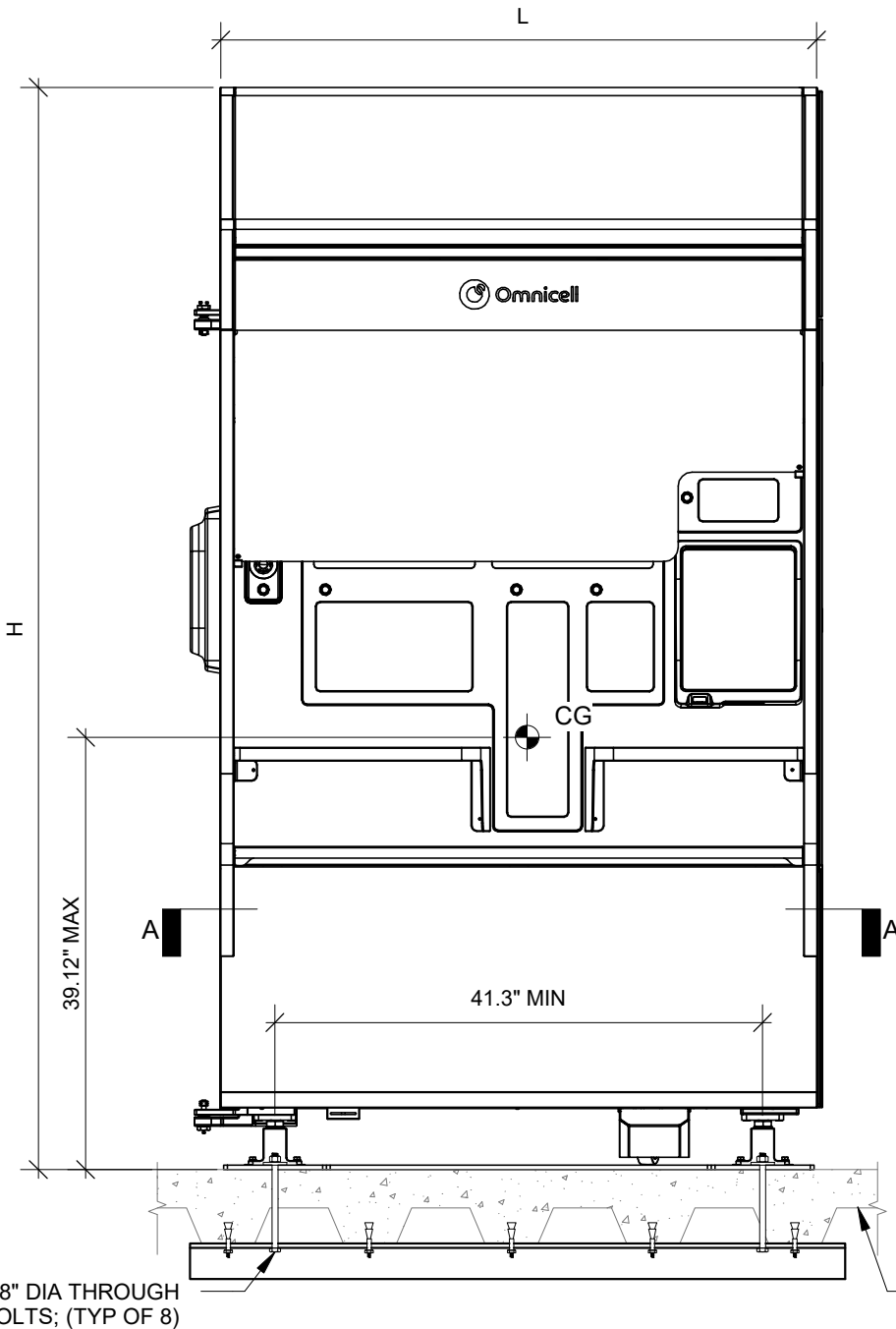
NOTES:

1. PROVIDE 3/8" HILTI KB-T22 W/ 1-1/2" EMBED TO PROVIDE SUPPORT TO ANGLE
2. PROVIDE 12" MINIMUM DISTANCE TO EDGE OF SLAB, OPENINGS OR OTHER ATTACHMENTS
3. PROVIDE HEX NUT AT TOP AND BOTTOM OF BOTTOM ANGLE FLANGE, TYP WHERE POSSIBLE
4. EXTEND THE BOTTOM ANGLE 2" PAST THE EDGE OF THE SLAB RIB TO INSTALL EXPANSION ANCHOR. DO NOT INSTALL EXPANSION ANCHOR IN THE SAME RIB AS THE THROUGH BOLT
5. ANCHORAGE DESIGN CONFORMS TO CBC 2022. FORCES GIVEN ARE AT THE STRENGTH LEVEL
6. SEE GENERAL NOTES SECTION ON PAGE 1 AND 2
7. SEE RESULTANT FORCES AND GEOMETRIC PROPERTIES OF THE EQUIPMENT ON PAGES 4-6
8. S.E.O.R. MAY RECALCULATE MAX. ANCHOR FORCES V_u AND T_u AT THEIR DISCRETION BASED ON PROJECT SPECIFIC DEMANDS
9. ALL HOLES THROUGH STEEL FOR BOLTS SHALL BE STANDARD SIZE HOLES PER AISC 15TH EDITION, TABLE J3.3 UNLESS OTHERWISE NOTED
10. FOR CONCRETE ELEVATED SLAB, PROVIDE BOTTOM ANGLE

MODELS

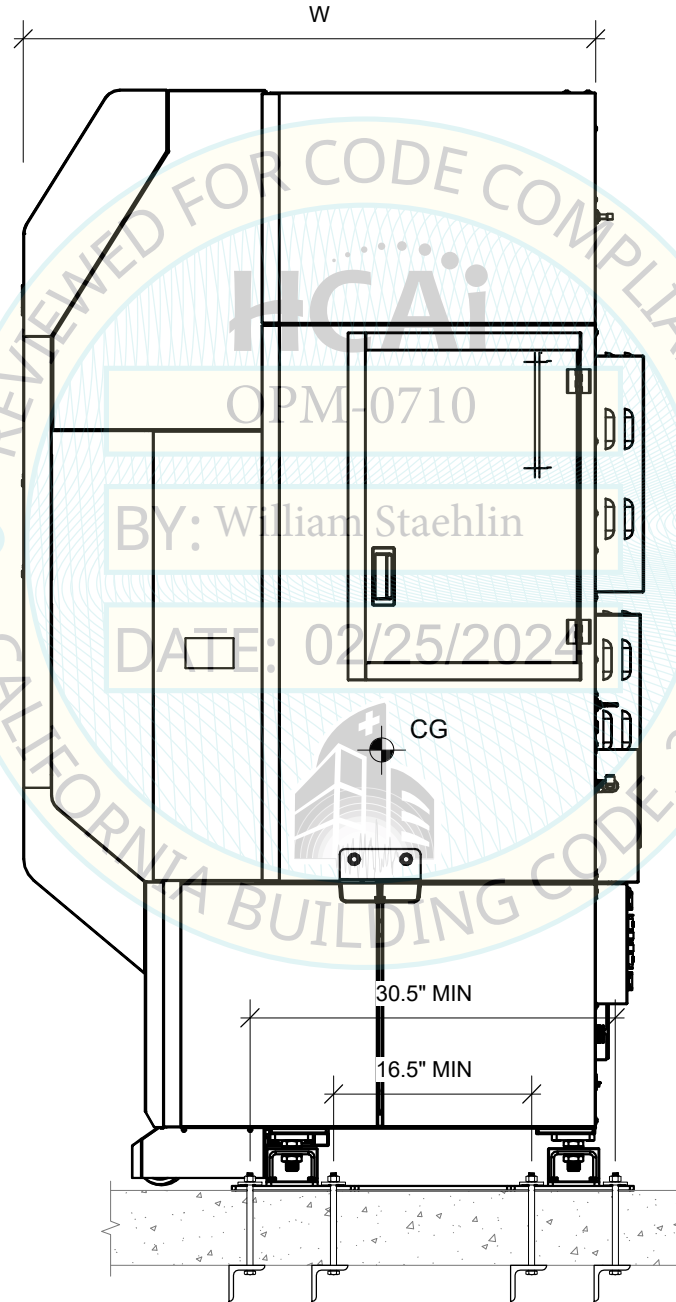
IVX STATION

CASE 1 - EQUIPMENTS ABOVE GRADE



FRONT ELEVATION

SEE "MINIMUM STEEL DECK REQUIREMENTS" DETAIL FOR SLAB PROPERTIES ON PAGE 4



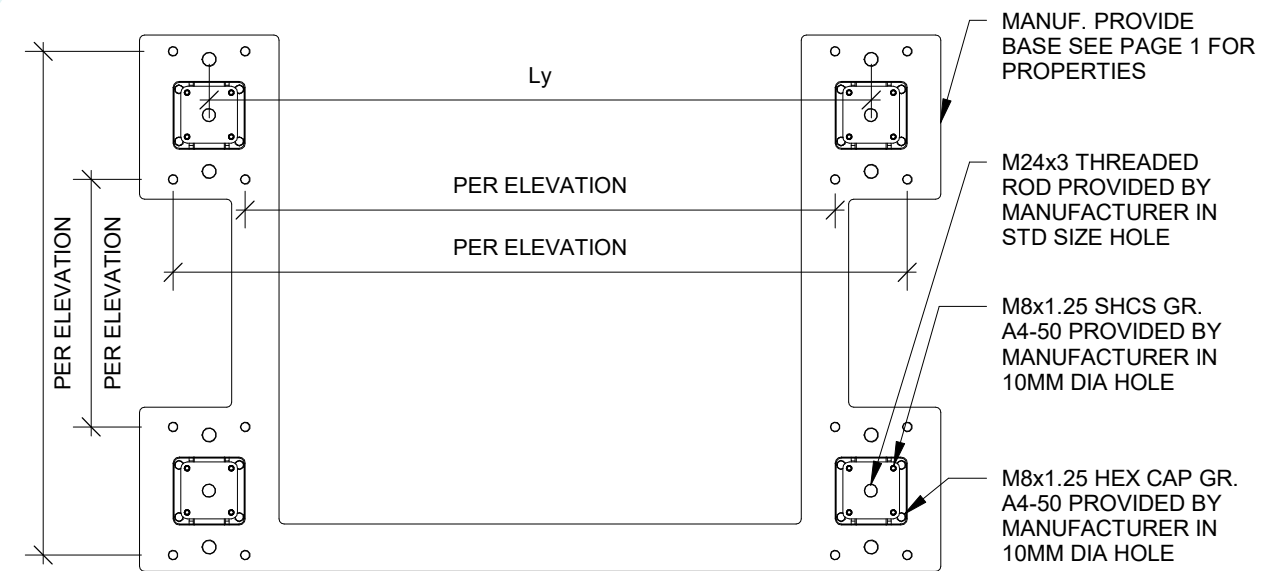
SIDE ELEVATION

NOTES:

- WHERE $z/h = 0$, THE DESIGN IS GOVERNED BY THE CAPACITY OF THE EXPANSION ANCHORS INTO CONCRETE.
- WHERE $z/h \leq 1$, THE DESIGN IS GOVERNED BY THE CAPACITY OF THE BASEPLATE MOUNT.
- T_{ult} , q AND V_{ult} SHOWN ON THE TABLE ARE THE MAXIMUM FORCES AT THE STRENGTH LEVEL AND HAVE NOT BEEN AMPLIFIED BY Ω_o . FOR ANCHORAGE TO CONCRETE, LOADS ARE REQUIRED TO BE AMPLIFIED BY Ω_o .
- PER DIAGRAM, NOTE THAT T_{ult} IS THE TENSION FORCE APPLIED TO EACH ANCHORS AND V_{ult} IS THE SHEAR FORCE APPLIED TO EACH ANCHOR
- BOLTS THROUGH CONCRETE ON METAL DECK
 - BOLTS SHALL BE TORQUED BY 3/4 TURN OF THE NUTS AFTER THE SNUG TIGHT CONDITION (THE SNUG TIGHT CONDITION IS DEFINED AS THE TIGHTNESS REQUIRED TO BRING THE CONNECTED PLIES INTO FIRM CONTACT) IS ACHIEVED.
 - THROUGH BOLTS IN CONCRETE SHALL RECEIVE SPECIAL INSPECTION AND TESTING IN ACCORDANCE WITH REQUIREMENTS FOR POST-INSTALLED ANCHORS.
- THE ANGLE WHICH THE MAXIMUM AXIAL FORCE DUE TO SEISMIC FORCE IS APPLIED IS q_{max} .

MODEL	W_p (LBS)	FORCES		L (IN)	W (IN)	H (IN)
		$\Omega_o V_{ult}$ (LBS)	T_{ult} (LBS)			
IVX	2118	497	1640	51.21	50.96	91.60

V_{ult} = MAXIMUM SHEAR PER THROUGH-BOLT AT STRENGTH LEVEL
 T_{ult} = MAXIMUM TENSION PER THROUGH-BOLT AT STRENGTH LEVEL
 W_p = TOTAL WEIGHT



PLAN SECTION A-A



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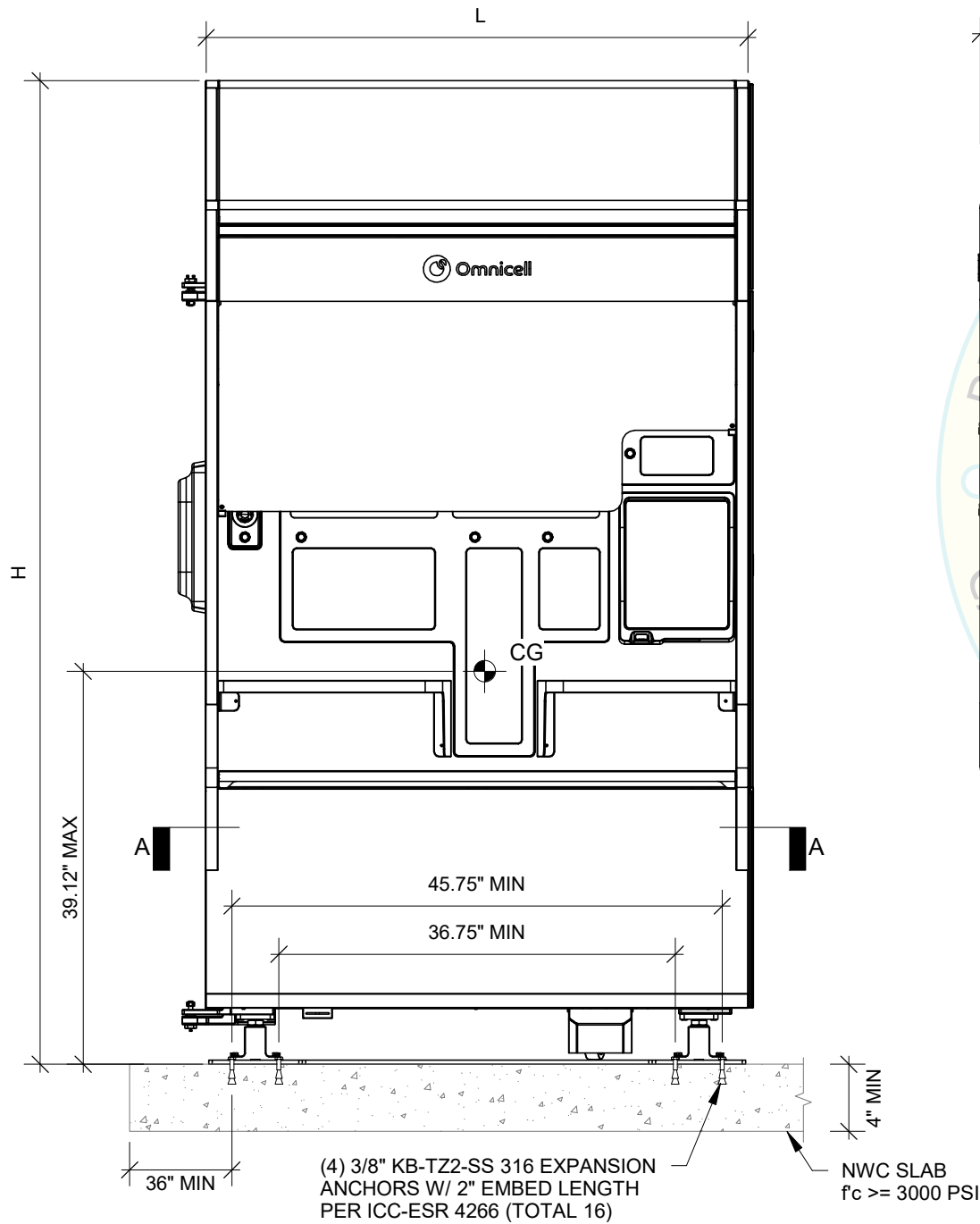
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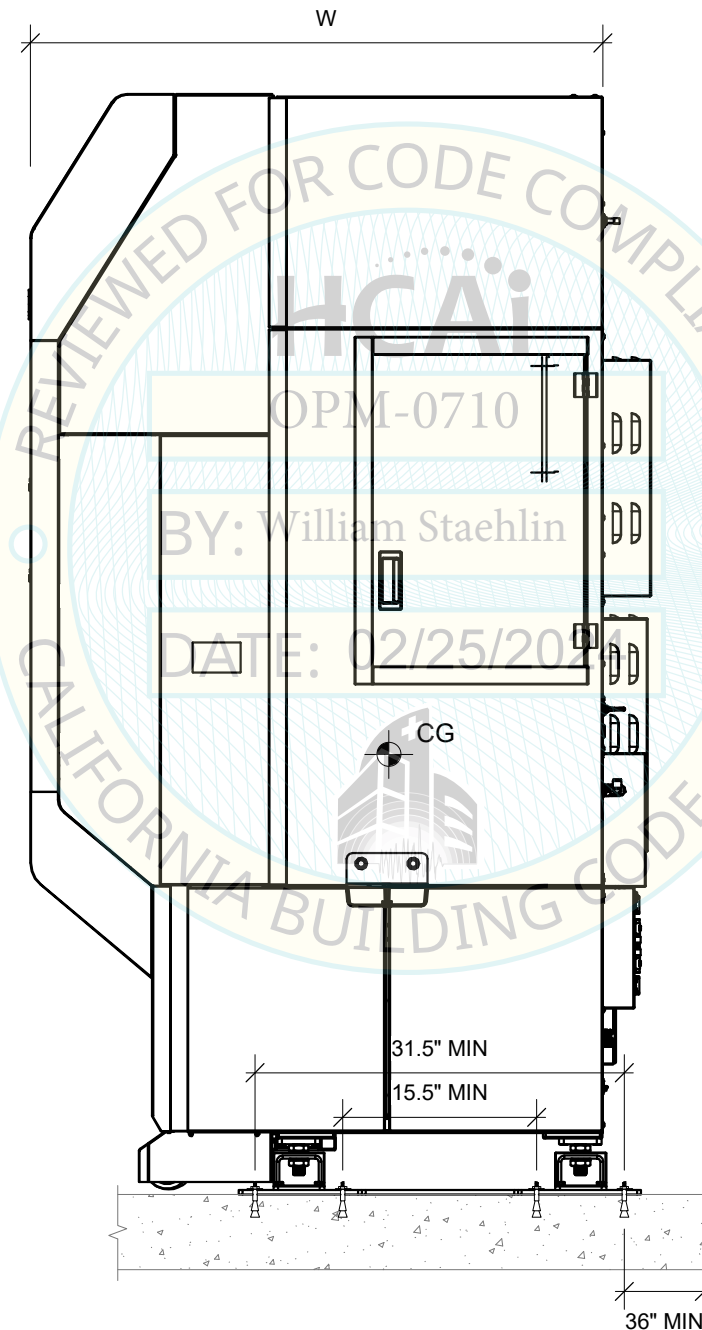
MODELS

IVX STATION

CASE 2 - EQUIPMENTS AT GRADE OR BELOW: 3/8" HILTI KB-TZ2 EMBED LENGTH = 2"



FRONT ELEVATION



SIDE ELEVATION

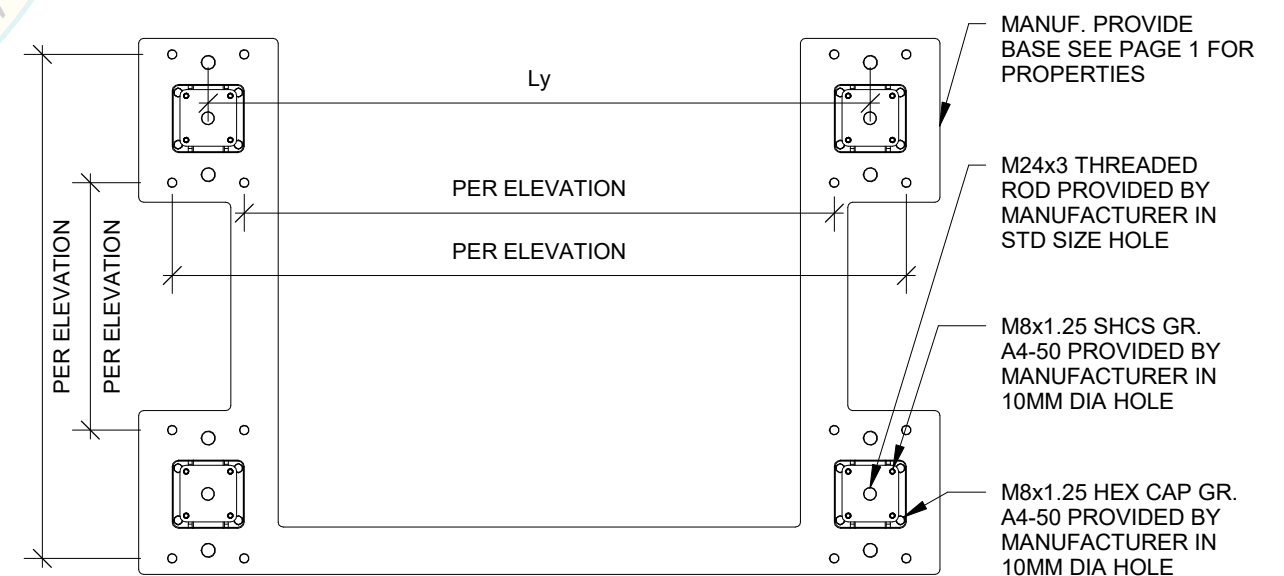
NOTES:

- WHERE $z/h = 0$, THE DESIGN IS GOVERNED BY THE CAPACITY OF THE EXPANSION ANCHORS INTO CONCRETE.
- WHERE $z/h \leq 1$, THE DESIGN IS GOVERNED BY THE CAPACITY OF THE BASEPLATE MOUNT.
- T_{ult} , q AND V_{ult} SHOWN ON THE TABLE ARE THE MAXIMUM FORCES AT THE STRENGTH LEVEL AND HAVE NOT BEEN AMPLIFIED BY Ω_o . FOR ANCHORAGE TO CONCRETE, LOADS ARE REQUIRED TO BE AMPLIFIED BY Ω_o .
- PER DIAGRAM, NOTE THAT T_{ult} IS THE TENSION FORCE APPLIED TO EACH ANCHORS AND V_{ult} IS THE SHEAR FORCE APPLIED TO EACH ANCHOR
- BOLTS THROUGH CONCRETE ON METAL DECK

- BOLTS SHALL BE TORQUED BY 3/4 TURN OF THE NUTS AFTER THE SNUG TIGHT CONDITION (THE SNUG TIGHT CONDITION IS DEFINED AS THE TIGHTNESS REQUIRED TO BRING THE CONNECTED PLIES INTO FIRM CONTACT) IS ACHIEVED.
 - THROUGH BOLTS IN CONCRETE SHALL RECEIVE SPECIAL INSPECTION AND TESTING IN ACCORDANCE WITH REQUIREMENTS FOR POST-INSTALLED ANCHORS.
6. THE ANGLE WHICH THE MAXIMUM AXIAL FORCE DUE TO SEISMIC FORCE IS APPLIED IS q_{max} .

MODEL	W_p (LBS)	FORCES		L (IN)	W (IN)	H (IN)
		ΩV_{ult}	ΩT_{ult}			
IVX	2118	379	1239	51.21	50.96	91.60

V_{ult} = MAXIMUM SHEAR PER ANCHOR AT STRENGTH LEVEL
 T_{ult} = MAXIMUM TENSION PER ANCHOR AT STRENGTH LEVEL
 W_p = TOTAL WEIGHT



PLAN SECTION A-A



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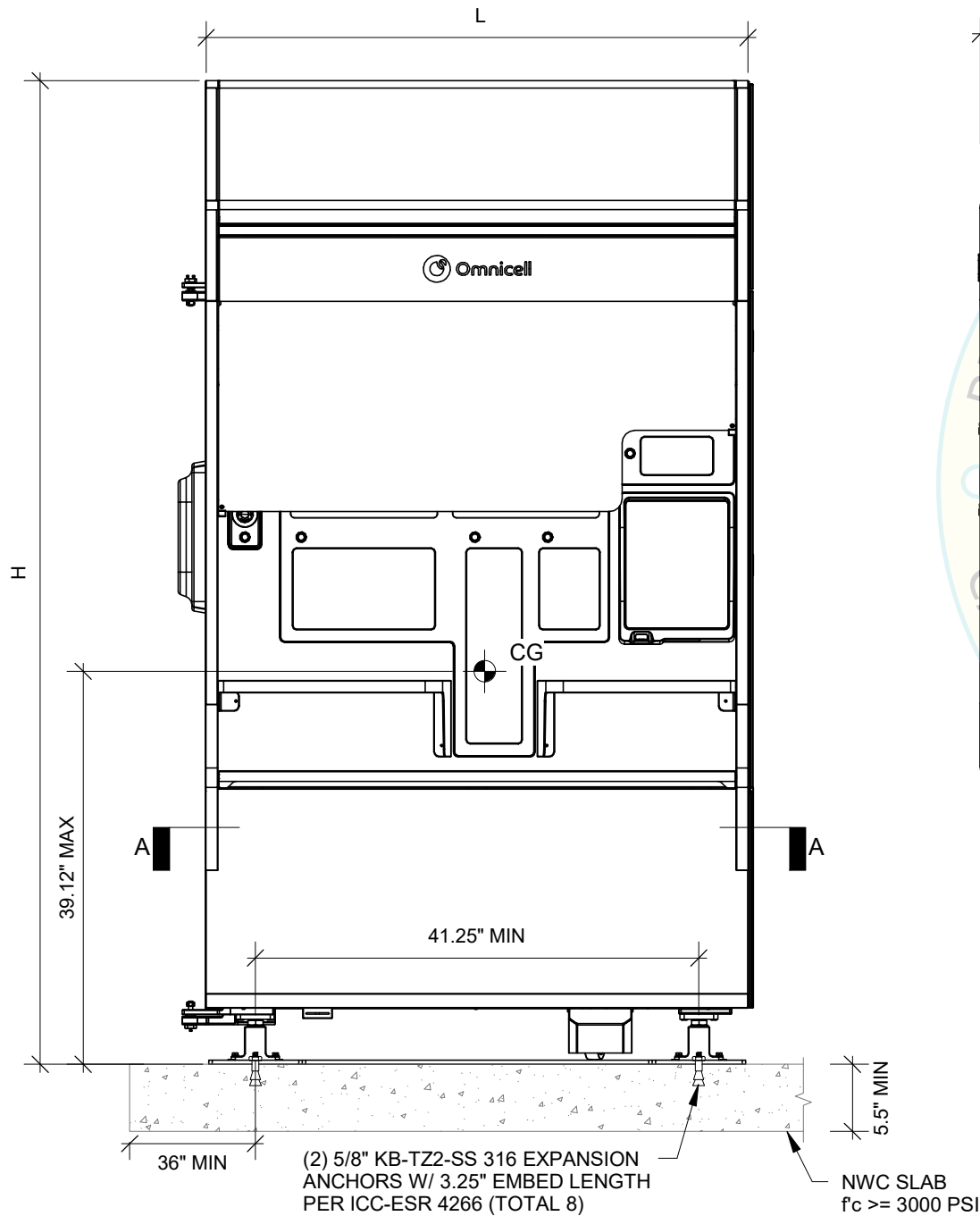
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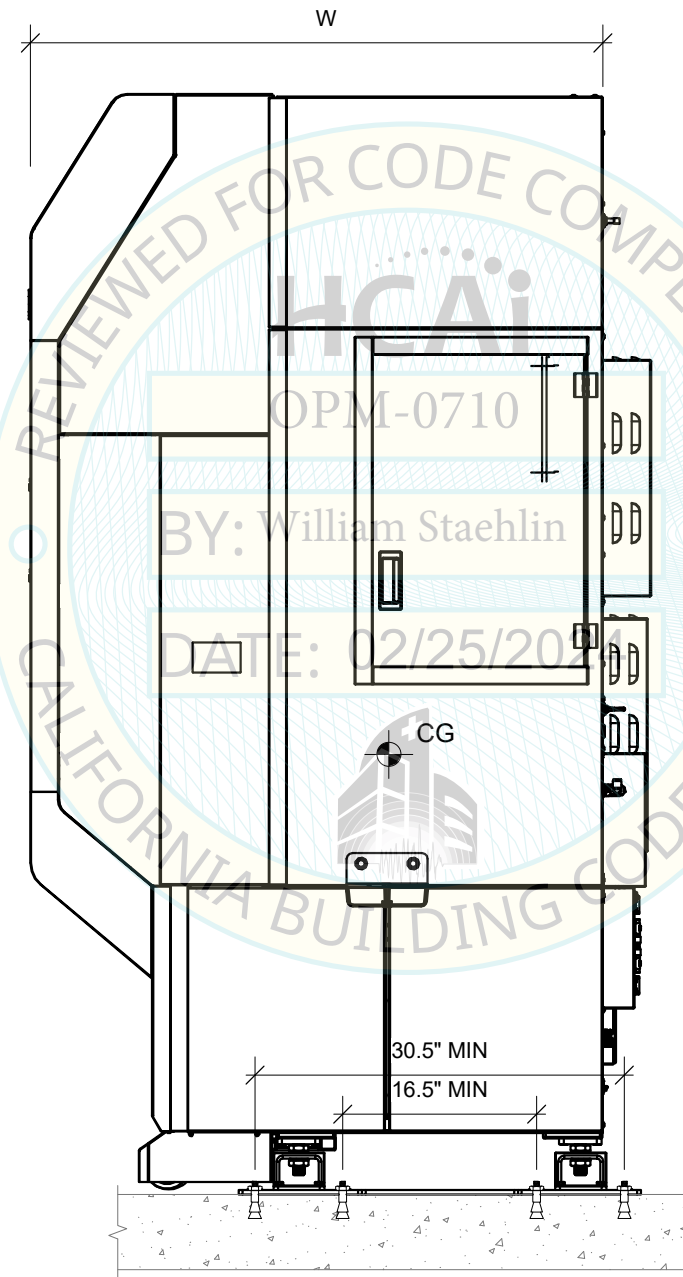
MODELS

IVX STATION

CASE 3 - EQUIPMENTS AT GRADE OR BELOW: 5/8" HILTI KB-TZ2 EMBED LENGTH = 3.25"



FRONT ELEVATION



SIDE ELEVATION

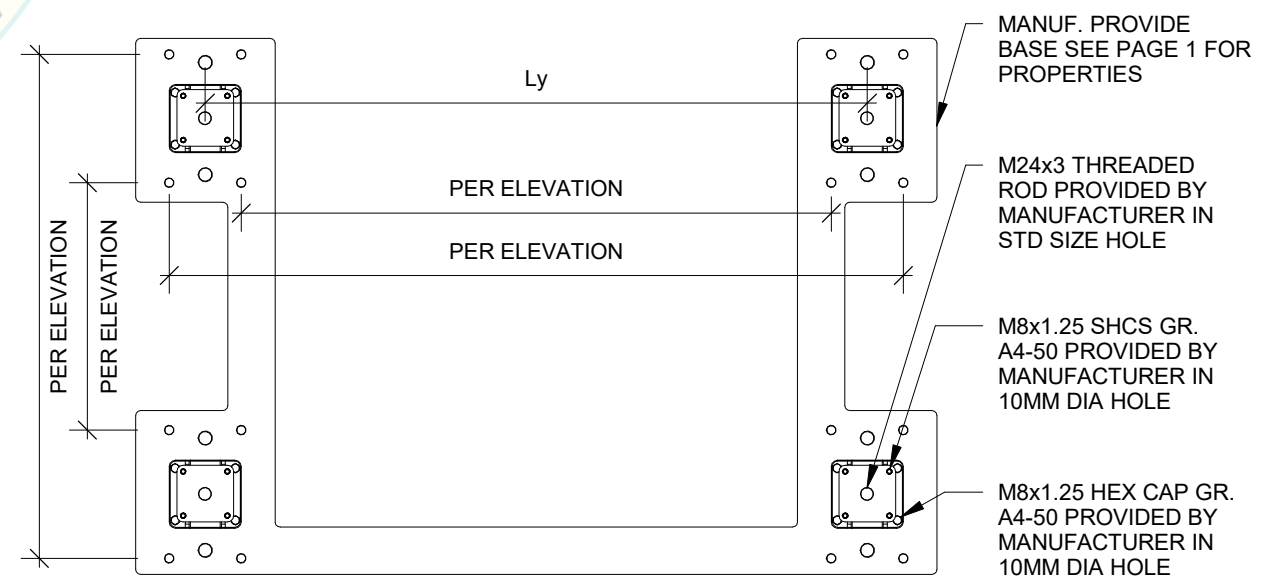
NOTES:

- WHERE $z/h = 0$, THE DESIGN IS GOVERNED BY THE CAPACITY OF THE EXPANSION ANCHORS INTO CONCRETE.
- WHERE $z/h \leq 1$, THE DESIGN IS GOVERNED BY THE CAPACITY OF THE BASEPLATE MOUNT.
- Tult, q AND Vult SHOWN ON THE TABLE ARE THE MAXIMUM FORCES AT THE STRENGTH LEVEL AND HAVE NOT BEEN AMPLIFIED BY Ω_o . FOR ANCHORAGE TO CONCRETE, LOADS ARE REQUIRED TO BE AMPLIFIED BY Ω_o .
- PER DIAGRAM, NOTE THAT Tult IS THE TENSION FORCE APPLIED TO EACH ANCHORS AND Vult IS THE SHEAR FORCE APPLIED TO EACH ANCHOR
- BOLTS THROUGH CONCRETE ON METAL DECK

- BOLTS SHALL BE TORQUED BY 3/4 TURN OF THE NUTS AFTER THE SNUG TIGHT CONDITION (THE SNUG TIGHT CONDITION IS DEFINED AS THE TIGHTNESS REQUIRED TO BRING THE CONNECTED PLIES INTO FIRM CONTACT) IS ACHIEVED.
 - THROUGH BOLTS IN CONCRETE SHALL RECEIVE SPECIAL INSPECTION AND TESTING IN ACCORDANCE WITH REQUIREMENTS FOR POST-INSTALLED ANCHORS.
6. THE ANGLE WHICH THE MAXIMUM AXIAL FORCE DUE TO SEISMIC FORCE IS APPLIED IS q_{max} .

MODEL	Wp (LBS)	FORCES		L (IN)	W (IN)	H (IN)
		ΩV_{ult} (LBS)	ΩT_{ult} (LBS)			
IVX	2118	745	2478	51.21	50.96	91.60

Vult = MAXIMUM SHEAR PER ANCHOR AT STRENGTH LEVEL
Tult = MAXIMUM TENSION PER ANCHOR AT STRENGTH LEVEL
Wp = TOTAL WEIGHT



PLAN SECTION A-A