DEPARTMENT OF HEALTH CARE ACCESS AND INFORMATION
OFFICE OF STATEWIDE HOSPITAL PLANNING AND DEVELOPMENT

APPLICATION FOR HCAI PREAPPROVAL OF MANUFACTURER'S CERTIFICATION (OPM)

OFFICE USE ONLY

APPLICATION #: OPM-0747

HCAI Preapproval of Manufacturer's Certification (OPM)

Type: [X] New [ ] Renewal/Update

Manufacturer Information

Manufacturer: M.W. Saussé & Co., Inc.

Manufacturer's Technical Representative: Nathan Tremblay

Mailing Address: 28744 Witherspoon Parkway, Valencia, CA 91355

Telephone: (661) 257-3311 Email: ntremblay@vibrex.net

Product Information

Product Name: RMU-EQ

Product Type: Floor Mounted Vibration Isolators With Restraints

Product Model Number: RMU-EQ-1, RMU-EQ-2 and RMU-EQ-3

General Description: Floor mounted vibration isolators with steel springs, neoprene elements, and integrated snubbers

Applicant Information

Applicant Company Name: M.W. Saussé & Co., Inc.

Contact Person: Nathan Tremblay

Mailing Address: 28744 Witherspoon Parkway, Valencia, CA 91355

Telephone: (661) 257-3311 Email: ntremblay@vibrex.net

Title: Principal Engineer

Registered Design Professonal Preparing Engineering Recommendations

Company Name: M.W. SAUSSE & COMPANY, INC

Name: Nathan Tremblay California License Number: S6481

Mailing Address: 28744 Witherspoon Parkway, Valencia, CA 913555425

Telephone: (661) 425-8061 Email: ntremblay@vibrex.net

HCAI Special Seismic Certification Preapproval (OSP)

☐ Special Seismic Certification is preapproved under OSP OSP Number:

Certification Method

Testing in accordance with: ☐ ICC-ES AC156 ☐ FM 1950-16

☒ Other(s) (Please Specify): ASHRAE 171 modified per HCAI/OSHPD (Draft dated March 2nd, 2023)

*Use of criteria other than those adopted by the California Building Standards Code, 2022 (CBSC 2022) for component supports and attachments are not permitted. For distribution system, interior partition wall, and suspended ceiling seismic bracings, test criteria other than those adopted in the CBSC 2022 may be used when approved by HCAI prior to testing.

☐ Analysis

☐ Experience Data

☐ Combination of Testing, Analysis, and/or Experience Data (Please Specify):

HCAI Approval

Date: 5/16/2025

Name: Timothy Piland Title: Senior Structural Engineer

Condition of Approval (if applicable):

vibration & seismic control systems

M.W. Saussé & Co., Inc.

28744 Witherspoon Parkway | Valencia, CA 91355

Ph: (661) 257-3311 | Fax: (661) 257-6050

SEISMICALLY RATED SPRING FLOOR MOUNTED VIBRATION ISOLATORS WITH RESTRAINTS

1st EDITION, 2025

OPM-0747-25

REGISTERED PROFESSIONAL ENGINEER
NATHAN D. TREMBLAY
No. S6481
Exp. 9/30/25
STRUCTURAL
STATE OF CALIFORNIA

1. General Notes & Seismic Capacity

a. HCAI Pre-Approval of Manufacturer's Certification is based on the 2022 & 2025 versions of the California Building Code (CBC).

b. The individual VIWR (vibration isolators with restraints) have been tested using a modified version of the ASHRAE 171, Method of Testing for Rating Seismic & Wind Restraints

c. For the design of the VIWR under the 2022 CBC, use the ASCE 7-16 Chapter 13 equations to calculate the lateral & vertical seismic loads and apply them at the top of the VIWR:

i. The horizontal seismic force, F_p

$\rightarrow F_p = \frac{0.4 a_p S_{DS} W_P}{\left(\frac{R_P}{I_p}\right)}\left(1+2\frac{z}{h}\right)$ Eqn. 13.3-1

$\rightarrow F_p = 1.6 S_{DS} I_p W_p$ Eqn. 13.3-2 (Upper Limit)

$\rightarrow F_p = 0.3 S_{DS} I_p W_p$ Eqn. 13.3-3 (Lower Limit)

Where per the ASCE 7-16 Table 13.6-1 for equipment using VIWR:

Amplification Factor, $a_p = 2.5$

Component Response Modification Factor, $R_p = 2.0$

Component Importance Factor, I_p:

For Risk Category IV projects & systems, $I_p = 1.5$

For Risk Category II and III projects & systems, $I_p = 1.0$

ii. The vertical seismic force, F_{pv}

$\rightarrow F_{pv} = \pm 0.2 S_{DS} W_p$ § 13.3.1.2

iii. The overstrength factor (Ω_0) per footnote c. of table 13.6-1 is for design of the anchorage to concrete & masonry and is not applicable for the design of the VIWR itself.

iv. Utilizing ASCE 7-16 § 2.3.6 (LRFD) & 2.4.5 (ASD)

$$\left.\begin{array}{l} 1.2D + 1.0Ev + 1.0Eh \\ 0.9D - 1.0Ev + 1.0Eh \end{array}\right\} \text{LRFD Eqns. 6 \& 7 per § 2.3.6}$$

$$\left.\begin{array}{l} 1.0D + 0.7Ev + 0.7Eh \\ 0.6D - 0.7Ev + 0.7Eh \end{array}\right\} \text{ASD Eqns. 8 \& 10 per § 2.4.5}$$

Where:

$D = W_p$ (Dead/operating load of equipment supported by VIWR)

$E_h = F_p$

$E_v = F_{pv}$

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v. Component seismic demands applied at the center of gravity (C.G.), relative to the center of rigidity (C.R.), using the basic principles of structural mechanics, P/A $\pm$ Mc/I in both orthogonal directions are resisted by the isolators (see 2a.ii) to determine the isolator vertical and lateral demands
For curbs:
$I = I_0 + Ad^2$ (Parallel Axis Theorem for rotation about edge of component)

d. For the design of the VIWR under the 2025 CBC, use the ASCE 7-22 Chapter 13 equations to calculate the lateral & vertical seismic loads and apply them at the top of the VIWR:

i. The horizontal seismic force, F_p

$\rightarrow F_p = 0.4 S_{DS} I_p W_p \left[\frac{H_f}{R_\mu}\right]\left[\frac{C_{AR}}{R_{po}}\right]$ Eqn. 13.3-1

$\rightarrow F_p = 1.6 S_{DS} I_p W_p$ Eqn. 13.3-2 (Upper Limit)

$\rightarrow F_p = 0.3 S_{DS} I_p W_p$ Eqn. 13.3-3 (Lower Limit)

For H_f, per ASCE 7-22 §13.3.1.1 use of either equation 13.3-4 or 13.3-5 is permitted:

$\rightarrow H_f = 1 + a_1\left(\frac{z}{h}\right) + a_2\left(\frac{z}{h}\right)^{10}$ Eqn. 13.3-4

$a_1 = 1/T_a \leq 2.5$

$a_2 = [1-(0.4/T_a)^2] \geq 0$

T_a = Lowest approximate fundamental period of the supporting structure below.

$\rightarrow H_f = 1 + 2.5\left(\frac{z}{h}\right)$ Eqn. 13.3-5

For R_μ, per ASCE 7-22 §13.3.1.2

$\rightarrow R_\mu = [1.1R/(I_e\Omega_0)]^{1/2} \geq 1.3$ Eqn. 13.3-6

I_e, R, & Ω_0 are design values used for the design of the main supporting building or non-building structure underneath the equipment.

And per the ASCE 7-22 Table 13.6-1 for equipment using VIWR:

Component Resonance Ductility Factor C_{AR}:

Supported at or below grade plane, $C_{AR} = 1.8$

Supported above grade plane by a structure, $C_{AR} = 2.2$

Component Strength Factor, $R_{po} = 1.3$

If data of the supporting building or non-building structure is not available, the following simplified version of Eqn. 13.3-1 may be used:

$\rightarrow F_p = 0.4 S_{DS} I_p W_p \left[\frac{1+2.5(z/h)}{1.3}\right]\left[\frac{C_{AR}}{R_{po}}\right]$

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Component Importance Factor, I_p:

For Risk Category IV projects & systems, $I_p = 1.5$

For Risk Category II and III projects & systems, $I_p = 1.0$

ii. The vertical seismic force, F_{pv}

$\rightarrow F_{pv} = \pm 0.2 S_{DS} W_p$ § 13.3.1.6

iii. The overstrength factor (Ω_0) per footnote b. of table 13.6-1 is for design of the anchorage to concrete & masonry and is not applicable for the design of the VIWR itself.

iv. Utilizing ASCE 7-22 § 2.3.6 (LRFD) & 2.4.5 (ASD)

$$\left.\begin{array}{l}1.2D + 1.0Ev + 1.0Eh \\ 0.9D - 1.0Ev + 1.0Eh\end{array}\right\}\text{ LRFD Eqns. 6 \& 7 per § 2.3.6}$$

$$\left.\begin{array}{l}1.0D + 0.7Ev + 0.7Eh \\ 0.6D - 0.7Ev + 0.7Eh\end{array}\right\}\text{ ASD Eqns. 8 \& 10 per § 2.4.5}$$

Where:

$D = W_p$ (Dead/operating load of equipment supported by VIWR)

$E_h = F_p$

$E_v = F_{pv}$

v. Component seismic demands applied at the center of gravity (C.G.), relative to the center of rigidity (C.R.), using the basic principles of structural mechanics, $P/A \pm Mc/I$ in both orthogonal directions are resisted by the isolators (see 2a.ii) to determine the isolator vertical and lateral demands

For curbs:

$I = I_0 + Ad^2$ (Parallel Axis Theorem for rotation about edge of component)

e. VIWR capacities are limited to the maximum calculated demands from 1c or 1d.

f. It is the responsibility of the Registered Design Professional (RDP) in responsible charge to submit to the Structural Engineer of record (SEOR) the following:

i. Project specific VIWR demands ≤ OPM listed capacities.

ii. Component attachment to the VIWR and VIWR attachment to the structure are in compliance with CBC and corresponding anchor attachment ICC Reports.

iii. Component assembly installation, e.g., component, VIWR, and attachments are in compliance with CBC and details within the OPM.

g. Environment factors, e.g., wind, snow, rain/floods, etc., are beyond the scope of the OPM.

REGISTERED PROFESSIONAL ENGINEER NATHAN D. TREMBLAY No. S6481 Exp. 9/30/25 STRUCTURAL STATE OF CALIFORNIA

2. General VIWR Design Procedure

a. The VIWR selection process is as follows:
   i. Based upon project requirements, select the appropriate VIWR family.
   ii. Lateral and vertical seismic demands are resisted per isolator.
   iii. Calculate the seismic demands for each isolator support the equipment per 1c or 1d.
   iv. For the selected isolator, the calculated seismic demand loads (uplift load and shear/horizontal load) must plot within the corresponding interaction curves on pages 1.5 (LRFD) & 1.6 (ASD).

3. Anchorage and Attachment Requirements

a. It is the responsibility of the Registered Design Professional (RDP) in responsible charge to submit to the Structural Engineer of Record (SEOR) the following:
   i. Equipment attachment to the isolator.
   ii. Anchorage of the isolator to building structure.

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Structural Engineer: N. Tremblay
California PE No. S6481

4. Tested Isolator Capacities

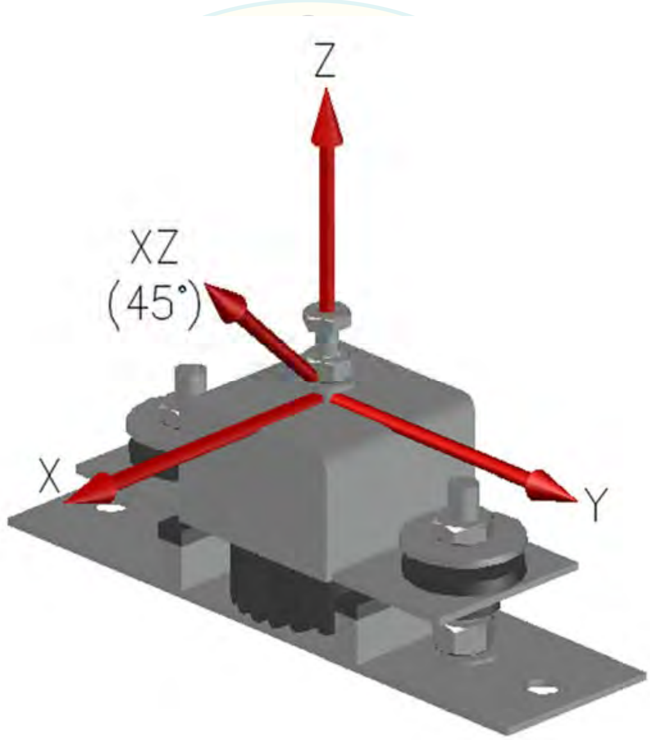
a. Nominal Tested Capacities

VIWR	R_x[1] (Lat. Shear)	R_y[2] (Lng. Shear)	R_{xz}[3] (45° Shear/Tens.)	Rz (Tension)
RMU-EQ-1	815	n/a	1,257	1,801
RMU-EQ-2	1,441	2,106	1,694	2,054
RMU-EQ-3	2,539	n/a	3,925	4,715

b. To use these tested values in the design of a system supporting MEP/Non-structural equipment, the following strength reduction factors have been applied to the values based on the selected design methodology:

i. LRFD or SD methodology: $\phi_i = 0.80$
   - $T_s = \phi_i R_z$ or $T_s = \Omega_i R_z$

ii. ASD: $\Omega_i = 1/1.8 = 0.56$



VIWR LOADING FIGURE

REGISTERED PROFESSIONAL ENGINEER
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No. S6481
Exp. 9/30/25
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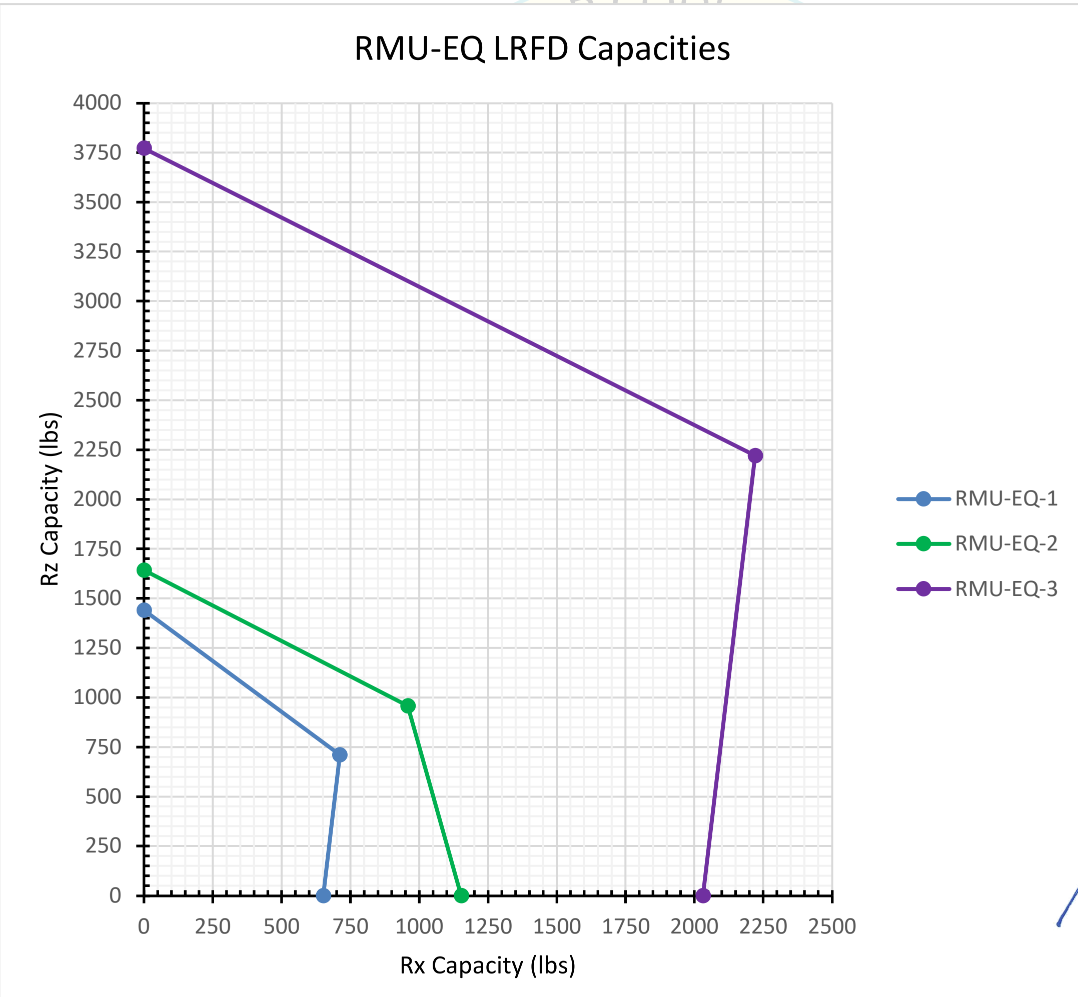
STATE OF CALIFORNIA

c. Design Capacity Graphs

LRFD DESIGN CAPACITIES (ϕ_i = 0.80)

VIWR	R_x[1] (Lat. Shear)	R_y[2] (Lng. Shear)	R_{xz}[3] (45° Shear/Tens.)	R_z (Tension)
RMU-EQ-1	652	n/a	1006	1441
RMU-EQ-2	1153	1685	1355	1643
RMU-EQ-3	2031	n/a	3140	3772

1. R_x is the transverse shear across the VIWR baseplate length.
2. R_y is the longitudinal shear along the VIWR baseplate length. Only RMU-EQ-2 was tested for R_y values for its use in cold formed sheet steel roof curb assemblies which utilize a wall action design approach (VIWRs parallel to the system lateral load (F_p) resist the shear loads while VIWRs perpendicular to the system shear force resist the uplift/downforce of the system lateral load (F_p)).
3. R_{xz} is the is the combined tension and lateral shear with the load applied to 45°. The load shown on the capacity graph for R_{xz} is R_z and R_x equal to 0.707*R_{xz}.
4. Tested load values are in pounds force.
5. See figure 1 below for typical axis orientation for a VIWR.



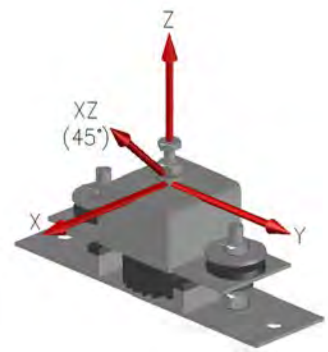


FIGURE 1

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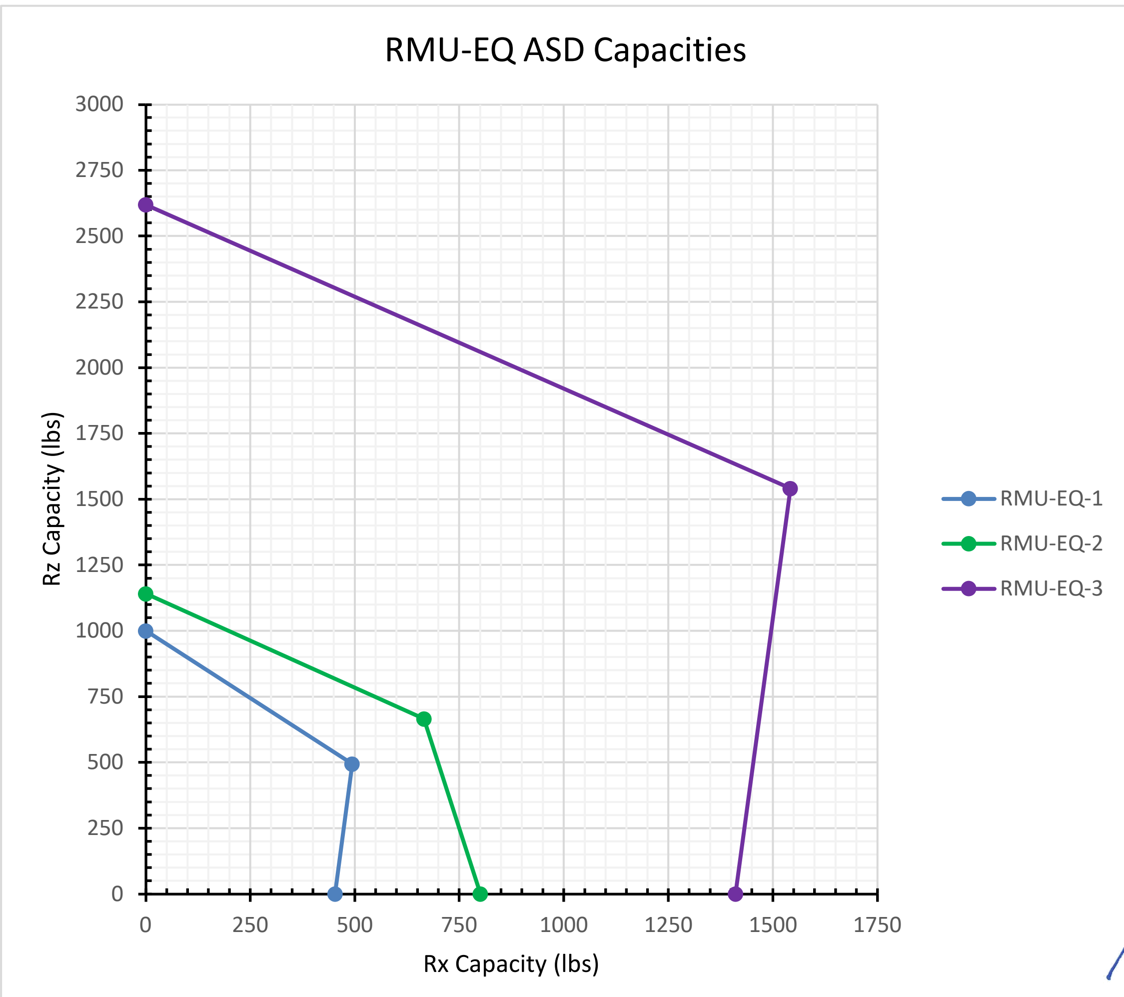
ASD DESIGN CAPACITIES (Ω_I = 1.80)

VIWR	R_x[1] (Lat. Shear)	R_y[2] (Lng. Shear)	R_{xz}[3] (45° Shear/Tens.)	R_z (Tension)
RMU-EQ-1	453	n/a	698	1001
RMU-EQ-2	801	1170	941	1141
RMU-EQ-3	1411	n/a	2181	2619

1. R_x is the transverse shear across the VIWR baseplate length.
2. R_y is the longitudinal shear along the VIWR baseplate length. Only RMU-EQ-2 was tested for R_y values for its use in cold formed sheet steel roof curb assemblies which utilize a wall action design approach (VIWRs parallel to the system lateral load (F_p) resist the shear loads while VIWRs perpendicular to the system shear force resist the uplift/downforce of the system lateral load (F_p)).
3. R_{xz} is the is the combined tension and lateral shear with the load applied to 45°. The load shown on the capacity graph for R_{xz} is R_z and R_x equal to 0.707*R_{xz}.
4. Tested load values are in pounds force.
   See figure 1 below for typical axis orientation for a VIWR.



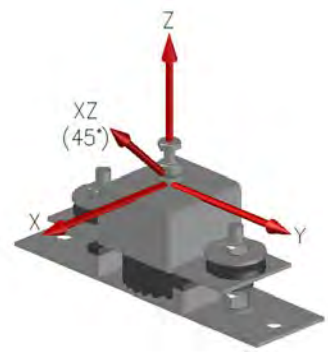


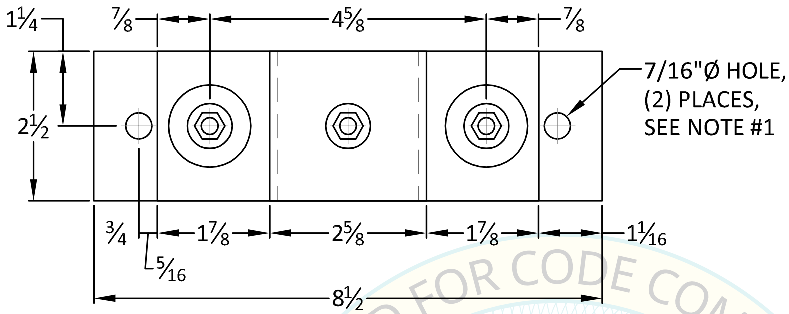
FIGURE 1

VIWR TYPE RMU-EQ-1

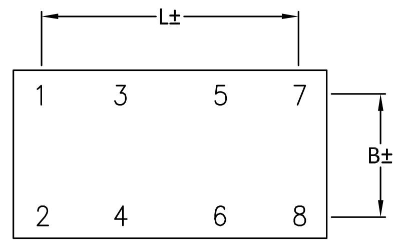
EQUIPMENT TAG	MAKE	MODEL	WEIGHT	L±	B±	B±

TYPE RMU–EQ–1 VIWR STATIC LOAD SPRING TABLE

SPRING	OD[2]	DEFL[3]	LOAD[4]	COLOR
B	2"	1"	25	BROWN
L	2"	1"	50	LAVENDER
G	2"	1"	75	GREEN
O	2"	1"	100	ORANGE
Y	2"	1"	130	YELLOW
W	2"	1"	185	WHITE
P	2"	1"	230	PINK
R	2"	1"	330	RED

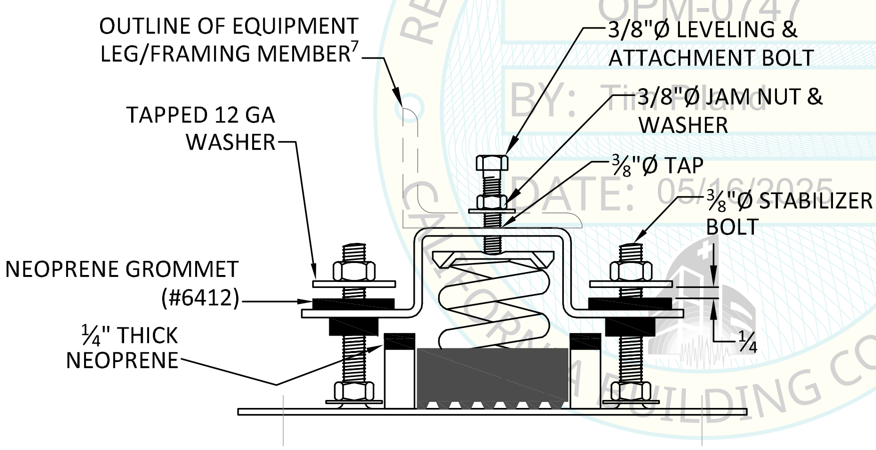


RMU–EQ–1 PLAN VIEW

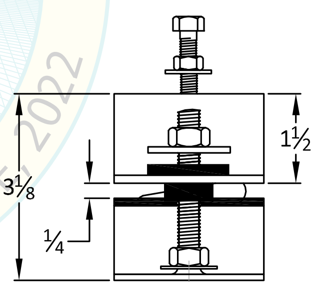


VIWR MOUNTING PLAN VIEW[5]

VIWR SPRING SELECTION	
1:	5:
2:	6:
3:	7:
4:	8:



RMU–EQ–1 FRONT ELEVATION



RMU–EQ–1 END ELEVATION

NOTES:

1. ATTACHMENT OF VIWR TO STRUCTURAL ELEMENTS BELOW BY REGISTERED DESIGN PROFESSIONAL.
   1.1. VIWR MAY BE WELDED TO STEEL BELOW IN LIEU OF FASTENERS/ANCHORS.
2. OD IS THE SPRING OUTER DIAMETER.
3. DESIGN DEFLECTION LISTED HAS A 50% RESERVE OF ADDITIONAL TRAVEL TO ACCOMODATE VARIANCE IN THE LOADS.
4. LOADS SHOWN ARE IN POUNDS.
5. FINISH: POWDER COATED HOUSING & SPRING W/ ZINC PLATED HARDWARE. CONTACT FACTORY FOR ADDITIONAL FINISH OPTIONS.
6. REFER TO INSTALLATION INSTRUCTIONS ON PAGE 2.3 FOR LEVELING AND ADJUSTMENT REQUIREMENTS.
7. DESIGN OF ADDITIONAL FRAMING SUPPORTING THE EQUIPMENT AND ATTACHED TO THE VIWR BY REGISTERED DESIGN PROFESSIONAL.

SEAL OF REGISTERED DESIGN PROFESSIONAL

Vibrex® vibration & seismic control systems

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Structural Engineer: N. Tremblay
California PE No. S6481

Page No.: **2.0**

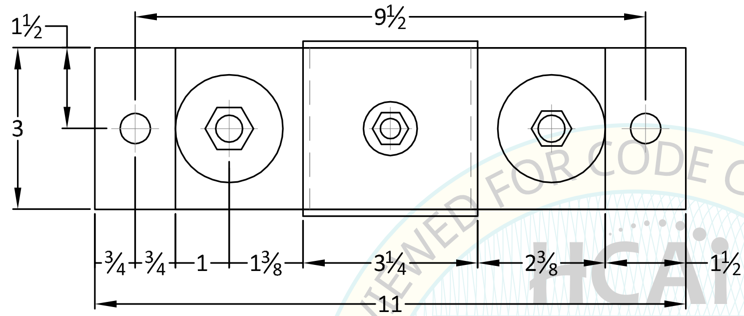
Date: **May 14, 2025**

VIWR TYPE RMU-EQ-2

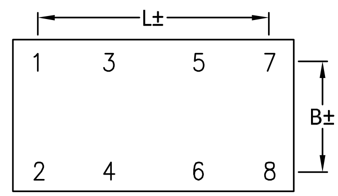
EQUIPMENT TAG	MAKE	MODEL	WEIGHT	L±	B±	B±

TYPE RMU-EQ-2 VIWR
STATIC LOAD SPRING TABLE

	SPRING	OD[2]	DEFL[3]	LOAD[4]	COLOR	SPRING	OD[2]	DEFL[3]	LOAD[4]	COLOR
	B	2"	1"	25	BROWN	2530	2.5"	2"	60	GOLD
	L	2"	1"	50	LAVENDER	2545	2.5"	2"	90	BLACK
	G	2"	1"	75	GREEN	2560	2.5"	2"	120	WHITE
	O	2"	1"	100	ORANGE	2588	2.5"	2"	175	YELLOW
	Y	2"	1"	130	YELLOW	25100	2.5"	2"	200	BLUE
	W	2"	1"	185	WHITE	25125	2.5"	2"	243	BROWN
	P	2"	1"	230	PINK	25150	2.5"	2"	293	RED
	R	2"	1"	330	RED	25175	2.5"	2"	329	PINK
INNER SPRINGS	1	1.42"	1"	40	LAVENDER	25225	2.5"	2"	400	PURPLE
INNER SPRINGS	2	1.42"	1"	100	BLUE	1925	1.9"	2"	50	SILVER
INNER SPRINGS	--	-	-	--	--	1975	1.9"	2"	143	GREEN
INNER SPRINGS	--	-	-	--	--	19150	1.9"	2"	263	ORANGE

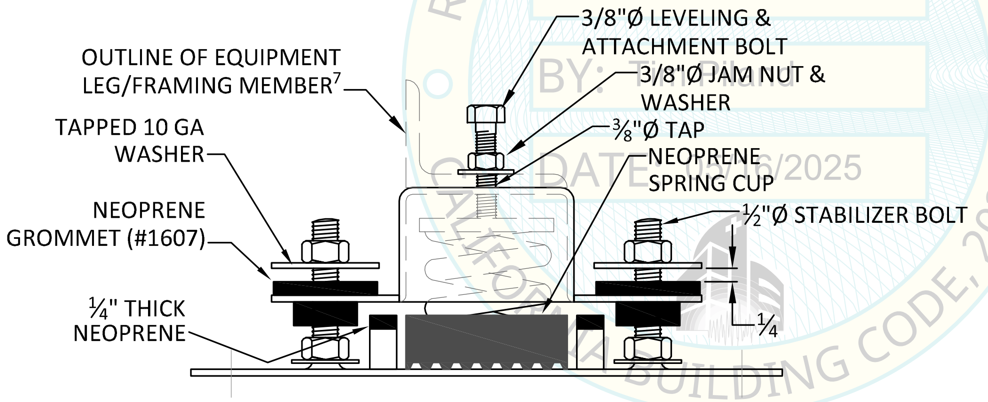


RMU-EQ-2 PLAN VIEW

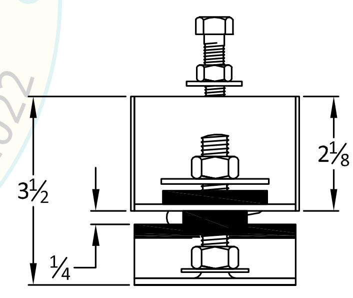


VIWR MOUNTING PLAN VIEW[5]

VIWR SPRING SELECTION	
1:	5:
2:	6:
3:	7:
4:	8:



RMU-EQ-2 FRONT ELEVATION



RMU-EQ-2 END ELEVATION

NOTES:

1. ATTACHMENT OF VIWR TO STRUCTURAL ELEMENTS BELOW BY REGISTERED DESIGN PROFESSIONAL.
   1.1. VIWR MAY BE WELDED TO STEEL BELOW IN LIEU OF FASTENERS/ANCHORS.
2. OD IS THE SPRING OUTER DIAMETER.
3. DESIGN DEFLECTION LISTED HAS A 50% RESERVE OF ADDITIONAL TRAVEL TO ACCOMODATE VARIANCE IN THE LOADS.
4. LOADS SHOWN ARE IN POUNDS AT THE DESIGN DEFLECTION.
5. FINISH: POWDER COATED HOUSING & SPRING W/ ZINC PLATED HARDWARE. CONTACT FACTORY FOR ADDITIONAL FINISH OPTIONS.
6. REFER TO INSTALLATION INSTRUCTIONS ON PAGE 2.3 FOR LEVELING AND ADJUSTMENT REQUIREMENTS.
7. DESIGN OF ADDITIONAL FRAMING SUPPORTING THE EQUIPMENT AND ATTACHED TO THE VIWR BY REGISTERED DESIGN PROFESSIONAL.

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Structural Engineer: N. Tremblay
California PE No. S6481

Page No.: 2.1

Date: May 14, 2025

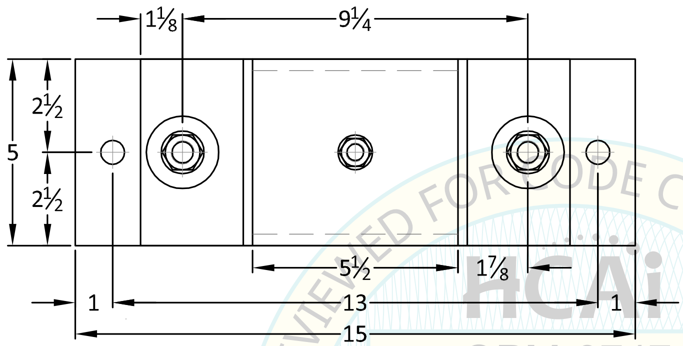
VIWR TYPE RMU-EQ-3

EQUIPMENT TAG	MAKE	MODEL	WEIGHT	L±	B±	B±

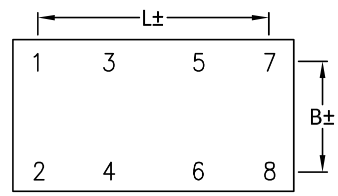
TYPE RMU-EQ-3 VIWR
STATIC LOAD SPRING TABLE

SPRING	OD[2]	DEFL[3]	LOAD[4]	COLOR	SPRING	OD[2]	DEFL[3]	LOAD[4]	COLOR
50	4"	2"	40	BLUE	56	4"	2"	540	YELLOW
51	4"	2"	80	BROWN	56A	4"	2"	700	WHITE
52	4"	2"	160	RED	57	4"	2"	800	PINK
53	4"	2"	240	LAVENDER	554	4"	2"	1100	XXX
54	4"	2"	320	GREEN	744	4"	2"	1480	XXX
55	4"	2"	400	ORANGE	804	4"	2"	1600	XXX

INNER SPRINGS	SPRING	OD[2]	DEFL[3]	LOAD[4]	COLOR
	59	2.8"	2"	400	BLUE
	60	2.8"	2"	800	XXX
	--	-	-	--	--
	--	-	-	--	--



RMU-EQ-3 PLAN VIEW



VIWR MOUNTING PLAN VIEW[5]

VIWR SPRING SELECTION	
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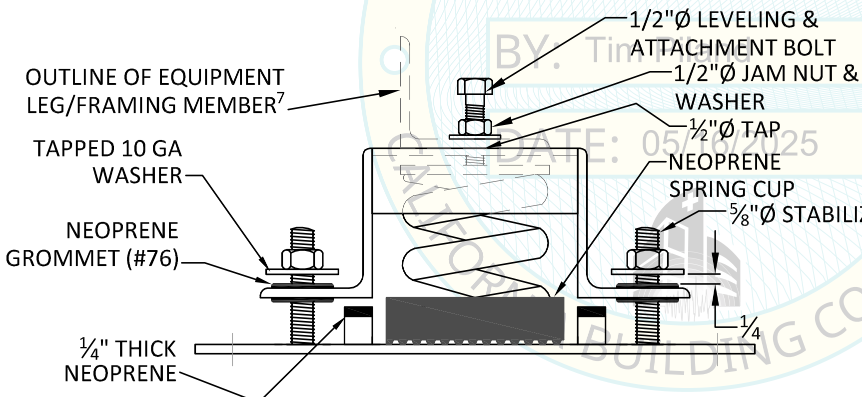
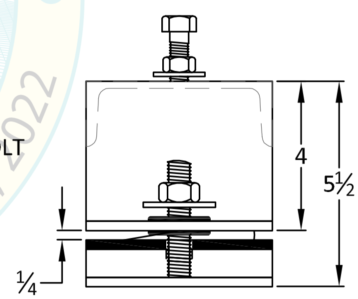
2:	6:
3:	7:
4:	8:



RMU-EQ-3 FRONT ELEVATION



RMU-EQ-3 END ELEVATION

NOTES:

1. ATTACHMENT OF VIWR TO STRUCTURAL ELEMENTS BELOW BY REGISTERED DESIGN PROFESSIONAL.
   1.1. VIWR MAY BE WELDED TO STEEL BELOW IN LIEU OF FASTENERS/ANCHORS.
2. OD IS THE SPRING OUTER DIAMETER.
3. DESIGN DEFLECTION LISTED HAS A 50% RESERVE OF ADDITIONAL TRAVEL TO ACCOMODATE VARIANCE IN THE LOADS.
4. LOADS SHOWN ARE IN POUNDS.
5. FINISH: POWDER COATED HOUSING & SPRING W/ ZINC PLATED HARDWARE. CONTACT FACTORY FOR ADDITIONAL FINISH OPTIONS.
6. REFER TO INSTALLATION INSTRUCTIONS ON PAGE 2.3 FOR LEVELING AND ADJUSTMENT REQUIREMENTS.
7. DESIGN OF ADDITIONAL FRAMING SUPPORTING THE EQUIPMENT AND ATTACHED TO THE VIWR BY REGISTERED DESIGN PROFESSIONAL.

REGISTERED PROFESSIONAL ENGINEER NATHAN D. TREMBLAY No. S6481 Exp. 9/30/25 STRUCTURAL STATE OF CALIFORNIA

SEAL OF REGISTERED DESIGN PROFESSIONAL

Vibrex® vibration & seismic control systems

M.W. Saussé & Co., Inc.
28744 Witherspoon Parkway | Valencia, CA 91355
Ph: (661) 257-3311 | Fax: (661) 257-6050

Structural Engineer: N. Tremblay
California PE No. S6481

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Date: May 14, 2025

VIBREX TYPE RMU-EQ
INSTALLATION INSTRUCTIONS

REGISTERED PROFESSIONAL ENGINEER NATHAN D. TREMBLAY No. S6481 Exp. 9/30/25 STRUCTURAL STATE OF CALIFORNIA

A) ISOLATORS ARE SHIPPED ASSEMBLED AND READY TO INSTALL.

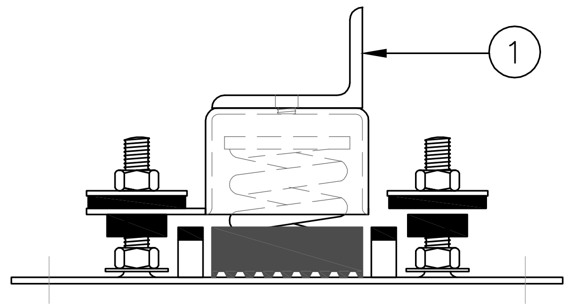
B) POSITION ISOLATORS UNDER UNIT MOUNTING LEGS (1) AT THEIR PROPER LOCATION. (VIEW A)

C) ATTACH UNIT TO ISOLATORS WITH LEVELING BOLT (2) (VIEW B)

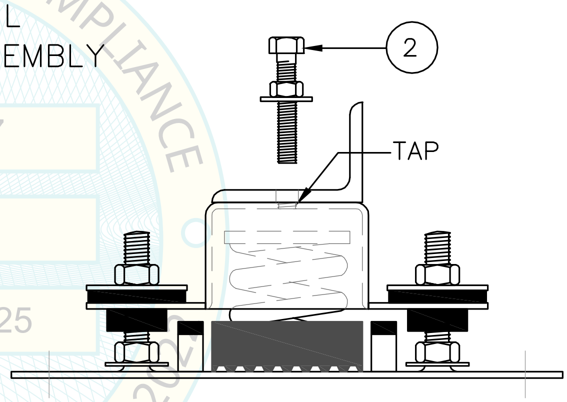
D) TO LEVEL THE EQUIPMENT, LOOSEN NUTS (3) AND (4) & THE TAPPED WASHER (5). TURN LEVELING BOLT (2) CLOCKWISE TO RAISE UNIT (VIEW C). LEVEL EACH ISOLATOR FOR THE EQUIPMENT INCREMENTALLY @ 1/4" TRAVEL (APPROXIMATELY 4 FULL TURNS) UNTIL A MAXIMUM 1/4" GAP BETWEEN THE TOP ASSEMBLY & LOWER BUMPERS IS ACHIEVED.

E) AFTER UNIT IS LEVEL, TIGHTEN NUT (4). MOVE NUT (3) & THE TAPPED WASHER (5) UP UNTIL A MAX 1/4" GAP BETWEEN THE WASHER & TOP ASSEMBLY IS ACHIEVED. LOCK THE WASHER (5) IN PLACE BY HOLDING W/ PLIERS & TIGHTEN THE NUT (3) HAND TIGHT. (VIEW D)

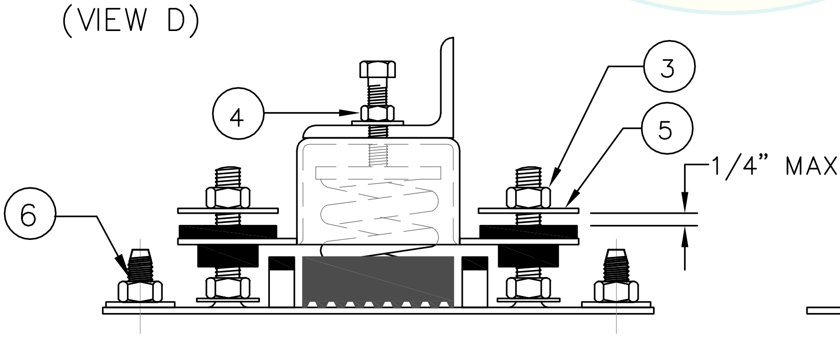
F) THE ISOLATOR SHOULD BE ANCHORED TO THE STRUCTURE WITH THE REQUIRED TYPE ANCHOR BOLTS (6) (BY OTHERS) ONLY AFTER ALL ISOLATORS ARE OPERATING PROPERLY! (VIEW D)



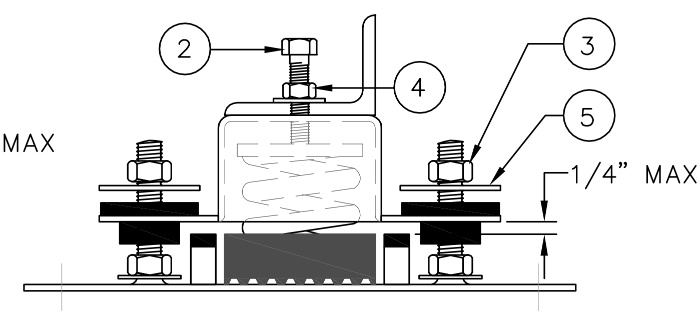
VIEW A



VIEW B



VIEW D



VIEW C

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