



DEPARTMENT OF HEALTH CARE ACCESS AND INFORMATION
OFFICE OF STATEWIDE HOSPITAL PLANNING AND DEVELOPMENT

APPLICATION FOR HCAI PREAPPROVAL OF
MANUFACTURER'S CERTIFICATION (OPM)

OFFICE USE ONLY

APPLICATION #: OPM-0761

HCAI Preapproval of Manufacturer's Certification (OPM)

Type: ☒ New ☐ Renewal/Update

Manufacturer Information

Manufacturer: Abatement Technologies

Manufacturer's Technical Representative: Adam Hunninck

Mailing Address: 1481 Harber Court, Fort Erie, ON L2A0G3 Canada

Telephone: (905) 871-4720

Email: ahunninck@abatement.ca

Product Information

Product Name: Shield Wall System

Product Type: Reusable modular wall panel system for containment solutions

Product Model Number: SW-CORE-P48; SW-P48W; SW-P48C; SW-CORE-P36; SW-P36W; SW-P36C; SW-CORE-P24; SW-P24W; SW-P24C; SW-CORE-P18; SW-P18W; SW-P18C; SW-CORE-P12; SW-P12W; SW-P12C; SW-CORE-EP24; SW-EP24W; SW-EP24C; SW-CORE-EP18; SW-EP18W; SW-EP18C; AGSHIELD-EXK; AGSHIELD-EX08; AGSHIELD-EX10; AGSHIELD-EX12; SW-CORE-D44; SW-D44W; SW-D44C; SW-CORE-D32; SW-D32W; SW-D32C; SW-CORE-DS42; SW-DS42W; SW-DS42C; SW-CORE-DS54; SW-DS54W; SW-DS54C; SW-CORE-DD68; SW-DD68W; SW-DD68C; SW-CFI; SW-CHU; SW-TI; SW-TI; AGSHIELD-WSD; AGSHIELD-WSA; AGSHIELD-HWPB; AGSHIELD-HWPB-10; AGSHIELD-HWCGB; AGSHIELD-HWPC3; AGSHIELD-HWPM1; AGSHIELD-HWPM2; AGSHIELD-CART; SKID

General Description: Reusable modular wall panels and components that can be pieced together to create interior barrier walls

Applicant Information

Applicant Company Name: Abatement Technologies

Contact Person: Adam Hunninck

Mailing Address: 1481 Harber Court, Fort Erie, ON L2A0G3 Canada

Telephone: (905) 871-4720

Email: ahunninck@abatement.ca

Title: Product Development Manager

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STATE OF CALIFORNIA – HEALTH AND HUMAN SERVICES AGENCY





DEPARTMENT OF HEALTH CARE ACCESS AND INFORMATION
OFFICE OF STATEWIDE HOSPITAL PLANNING AND DEVELOPMENT

Registered Design Professional Preparing Engineering Recommendations

Company Name: DEGENKOLB ENGINEERS

Name: Chad Closs

California License Number: S5946

Mailing Address: 225 Broadway, Suite 1325, San Diego, CA 92101

Telephone: (858) 699-5412

Email: ccloss@degenkolb.com

Certification Method

Testing in accordance with: ☐ ICC-ES AC156 ☐ FM 1950 ☐ ASHRAE 171 ☐ FEMA 461

☒ Other(s) (Please Specify): Testing in accordance to ASTM E72-22

*Use of criteria other than those adopted by the California Building Standards Code, 2025 and 2022 (CBSC 2025 and 2022) for component supports and attachments are not permitted. For distribution system, interior partition wall, and suspended ceiling seismic bracings, test criteria other than those adopted in the CBSC 2025 and 2022 may be used when approved by HCAI prior

☒ Analysis

☒ Experience Data

☒ Combination of Testing, Analysis, and/or Experience Data (Please Specify): Wall panels and interconnections certified

HCAI Approval

Date: 8/3/2026

Name: Roy Lobo

Title: Principal Structural Engineer

Condition of Approval (if applicable):

"A healthier California where all receive equitable, affordable, and quality health care"

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GENERAL NOTES

- I. GENERAL
1. THE SYSTEM DESCRIBED IN THESE DRAWINGS IS FOR TEMPORARY USE ONLY. THIS OSHPD PRE-APPROVAL OF MANUFACTURE’S CERTIFICATION (OPM) IS BASED ON THE CBC 2025. THE DEMAND (DESIGN FORCES) FOR USE WITH THIS OPM SHALL BE BASED ON THE CBC 2025.

2. THIS PRE-APPROVAL IS VALID FOR THE SYSTEM DESCRIBED IN THESE DRAWINGS THROUGHOUT THE STATE OF CALIFORNIA, AND IS VALID FOR INTERIOR WALLS INSTALLED AT ANY HEIGHT WITHIN THE BUILDING. SEE Cpm LIMITATIONS ON SHEET S2 FOR INTERIM SYSTEMS IN PLACE LONGER THAN 180 DAYS, THE WALL SHALL BE REMOVED PRIOR TO BEING GRANTED OCCUPANCY BY HCAI
- II. RESPONSIBILITIES OF THE STRUCTURAL ENGINEER OF RECORD
1. VERIFY MATERIALS AND WORKMANSHIP TO CONFORM WITH THE 2025 EDITION OF THE CALIFORNIA BUILDING CODE AND THE REQUIREMENTS OF THIS PRE-APPROVAL DOCUMENT.

2. VERIFY THE ADEQUACY OF THE EXISTING FRAMING TO SUPPORT THE LOADS INDICATED ON THIS SHEET, IN ADDITION TO ALL OTHER LOADS.

3. VERIFY ANCHORS ARE AT ADEQUATE DISTANCES FROM OPENINGS AND EDGES OF SLABS AS NOTED IN THE GENERAL NOTES SECTION IV.

4. VERIFY ANCHORS ARE AT ADEQUATE DISTANCES FROM NEW OR EXISTING ANCHORS AS NOTED IN THE GENERAL NOTES SECTION IV.

5. DESIGN ANY SUPPLEMENTARY MEMBER AND THEIR ATTACHMENTS OTHER THAN THOSE DETAILED WITHIN THIS PRE-APPROVAL.

6. VERIFY THAT THE INSTALLATION IS IN CONFORMANCE WITH THE 2025 CBC AND WITH THE DETAILS SHOWN IN THIS PRE-APPROVAL.

7. VERIFY THAT THE SEISMIC PARAMETERS DON’T EXCEED WHAT IS PERMITTED UNDER THIS OPM
- III. COLD-FORMED METAL FRAMING
1. STUDS: ASTM C955 AND ASTM A1003, “C” SHAPED WITH LIPPED FLANGES AND PUNCHED WEB. PROVIDE G60 COATING MINIMUM.

A. 43 MIL (18 GAGE) AND LIGHTER: GRADE 33 TYPE H

B. 54 MIL (16 GAGE) AND HEAVIER: GRADE 50 TYPE H

2. TRACK: ASTM C955 AND ASTM A1003, “U” SHAPED WITH UN-PUNCHED WEB. PROVIDE G60 COATING MINIMUM.

A. MATCH DEPTH, THICKNESS AND GRADE OF STUDS.

3. FRAMING DESIGNATIONS ON PLANS ARE BASED ON THE STEEL STUD MANUFACTURER’S ASSOCIATION (SSMA) PRODUCT TECHNICAL GUIDE (ICC-ESR-3064P).

4. INSTALL STUDS IN ACCORDANCE WITH MANUFACTURER’S INSTRUCTIONS AND ASTM C1007.

5. SHEET METAL SCREWS: SELF-DRILLING, SELF-TAPPING, HDG PER ASTM A153. PAN OR HEX WASHER HEAD AS REQUIRED BY FINISH.

A. PRODUCTS: ITW-BUILDEX TEKS SELECT (ICC-ESR-3223), GRABBER DRIVALL (ICC-ESR-1271)

6. MINIMUM SCREW SPACING AND EDGE DISTANCE TO BE 3/4".

7. POWDER ACTUATED FASTENERS: HILTI LOW-VELOCITY FASTENERS (ICC-ESR-2269).
- | BASE MATERIAL | FASTENERS | MINIMUM EMBEDMENT PER MANUF | MINIMUM EDGE DISTANCE | MINIMUM SPACING |
|---------------|-----------|-----------------------------|-----------------------|-----------------|
| STEEL | HILTI X-U | PER MANUF | 1/2" | |
| CONCRETE | HILTI X-P | 1" | 3" | 5 1/2" |
8. PAF SHALL NOT BE USED IN PRE-STRESSED CONCRETE UNLESS NON-DESTRUCTIVE TESTING METHODS ARE USED TO LOCATE STRAND AND REINFORCEMENT PRIOR TO FASTENER INSTALLATION.
9. TENSION TESTING IS NOT REQUIRED FOR POWDER ACTUATED FASTENERS USED TO ATTACH TRACKS OF INTERIOR NON-SHEAR WALL PARTITIONS FOR SHEAR ONLY, WHERE THERE ARE AT LEAST THREE FASTENERS PER SEGMENT OF TRACK.

- IV. MECHANICAL ANCHORS
1. EXPANSION OR WEDGE ANCHORS INTO CONCRETE: HILTI KB-TZ2 (ICC ESR-4266), SIMPSON STRONG-BOLT 2 (ICC-ESR-3037) OR DEWALT POWER-STUD+ SD2 (ICC-ESR-2502).

2. INSTALL ANCHORS IN ACCORDANCE WITH LATEST ICC-ESR REPORT AND MANUFACTURER INSTRUCTIONS.

3. WHEN INSTALLING DRILLED-IN ANCHORS AND/OR POWDER DRIVEN PINS IN (E) NON-PRESTRESSED REINFORCED CONCRETE, USE CARE AND CAUTION TO AVOID CUTTING OR DAMAGING THE (E) REINFORCING BARS. WHEN INSTALLING THEM INTO (E) PRESTRESSED CONCRETE (PRE- OR POST-TENSIONED) LOCATE THE PRESTRESSED TENDONS BY USING A NON-DESTRUCTIVE METHOD PRIOR TO INSTALLATION. EXERCISE EXTREME CARE AND CAUTION TO AVOID CUTTING OR DAMAGING THE TENDONS DURING INSTALLATION. MAINTAIN A MINIMUM CLEARANCE OF ONE INCH OR 3 ANCHOR DIAMETERS, WHICHEVER IS GREATER, BETWEEN THE REINFORCEMENT AND THE DRILLED-IN ANCHOR AND/OR PIN.

4. IF REINFORCEMENT IS ENCOUNTERED DURING DRILLING, ABANDON AND SHIFT THE HOLE LOCATION TO AVOID THE REINFORCEMENT. PROVIDE A MINIMUM OF 2 ANCHOR DIAMETERS OR 1 INCH, WHICHEVER IS LARGER, OF SOUND CONCRETE BETWEEN THE DOWEL AND THE ABANDONED HOLE. FILL THE ABANDONED HOLE WITH NON-SHRINK GROUT APPROVED BY THE STRUCTURAL ENGINEER OF RECORD. NOTIFY THE STRUCTURAL ENGINEER OF RECORD IF ANY REINFORCING IS DAMAGED.

5. ANCHORS WILL BE PROOF-TESTED BY OWNER’S TESTING AND INSPECTION AGENCY. WITH A REPORT OF THE TEST RESULTS SUBMITTED TO THE OWNER, IOR & SEOR.

6. IF ANY ANCHOR FAILS TESTING, REPLACE ANCHOR AND TEST ADDITIONAL ANCHORS OF THE SAME CATEGORY NOT PREVIOUSLY TESTED UNTIL TWENTY (20) CONSECUTIVE PASS, THEN RESUME INITIAL TESTING FREQUENCY.

7. TEST ANCHORS NO SOONER THAN 24 HOURS AFTER INSTALLATION.

8. TEST WEDGE ANCHORS PER THE FOLLOWING METHOD:

A. TORQUE WRENCH METHOD: TEST ANCHORS TO THE TORQUE LOAD INDICATED IN THE TABLE BELOW WITHIN THE FOLLOWING LIMITS:

1. ONE-HALF TURN OF THE NUT.
- | WEDGE-TORQUE LOAD (FT-LBS) | | | |
|----------------------------|--------------|--------------|--------------|
| ANCHOR DIA. (IN) | ICC ESR 4266 | ICC ESR 3037 | ICC ESR 2502 |
| 3/8
(CARBON STEEL) | 30 | 30 | 20 |
8. FOR POST INSTALLED ANCHORS USED FOR NONSTRUCTURAL APPLICATIONS, 50 PERCENT OR ALTERNATE BOLTS IN A GROUP, INCLUDING AT LEAST ONE-HALF THE ANCHORS IN EACH GROUP, SHALL BE TESTED
- WHERE POST-INSTALLED ANCHORS ARE USED FOR SILL PLATE BOLTING APPLICATIONS, 10 PERCENT OF THE ANCHORS SHALL BE TESTED.
9. MINIMUM EDGE DISTANCE:

A. 3/8" EXPANSION ANCHOR = 6"
10. MINIMUM SPACING (FROM NEW OR EXISTING ADJACENT ANCHORS):

A. 3/8" EXPANSION ANCHOR = 6"
11. IF ANY ANCHOR FAILS TESTING, ALL OF ANCHORS OF THE SAME TYPE SHALL BE TESTED, WHICH ARE INSTALLED BY THE SAME TRADE, NOT PREVIOUSLY TESTED UNTIL TWENTY (20) CONSECUTIVE ANCHORS PASS, THEN RESUME THE INITIAL TEST FREQUENCY.

- VI. STRUCTURAL TESTS, INSPECTIONS, AND OBSERVATIONS
1. AN INDEPENDENT APPROVED TESTING AGENCY AND SPECIAL INSPECTORS, CONFORMING TO 2025 CBC SECTION 1703A, WILL BE RETAINED BY THE OWNER TO PERFORM THE FOLLOWING TESTS AND INSPECTIONS. PROVIDE ACCESS AND FURNISH SAMPLES TO THE AGENCY AS REQUIRED.

2. THE FOLLOWING ITEMS REQUIRE TESTS AND INSPECTIONS IN ACCORDANCE WITH THE REQUIREMENTS OF THE CHAPTER "STRUCTURAL TESTS AND INSPECTIONS" OF THE CODE.

3. MECHANICAL ANCHORS:

A. VERIFY TYPE OF ANCHOR, ANCHOR DIMENSIONS, CONCRETE TYPE AND COMPRESSIVE STRENGTH, PREDRILLED HOLE DIMENSIONS, ANCHOR SPACING, EDGE DISTANCE, SLAB THICKNESS AND ANCHOR EMBEDMENT.

B. PROOF-TEST AS INDICATED IN THE MECHANICAL ANCHORS SECTION OF THESE GENERAL NOTES.
- VII. DESIGN CRITERIA
1. APPLICABLE CODE: 2025 CALIFORNIA BUILDING CODE.

2. SEISMIC DESIGN:

SEISMIC FORCE F (LRFD) = $0.4 * S_{DS} * I_p [H_f/R_{\mu}] [C_{AR}/R_{po}] W_p$

WHERE:

S_{DS} = 2.5

MAX ACCEL CONSIDERED

I_p = 1.5

Cpm COEFFICIENT = F_p/W_p

COMPONENT PROPERTIES (INTERIOR PARTITION WALLS > 9FT HT)

C_{AR} = 1.4

R_{po} = 1.5

Ω = 2.0

STRUCTURE PROPERTIES (VARIES, WORST CASE RESULTS IN Cpm = 3.8)

H_f = FORCE AMPLIFICATION FACTOR (FUNCTION OF HT IN STRUCTURE)

R_{μ} = STRUCTURE DUCTILITY REDUCTION FACTOR
- *ASCE 7-16 CAN ALSO BE USED TO CALCULATE THE Cpm COEFFICIENT, AND THE APPLICABLE VALUES CAN THEN BE REFERENCED DIRECTLY ON SHEET S2
- VIII. HOW TO USE THIS PRE-APPROVAL
1. REVIEW AND UNDERSTAND ALL GENERAL NOTES AND FIGURES BEFORE PROCEEDING.

2. FOR THE TYPICAL PARTITION WALL CONDITION AND SEISMICITY (Cpm):

A. DETERMINE THE TOP TRACK CONDITION AND BRACE SPACING FROM THE TABLES ON S2 AND BRACE LAYOUT ON S3.

B. BASED ON THE STRUCTURE TYPE, SELECT A BRACE CONNECTION AND WALL BASE CONNECTION FROM THE TABLE ON S4.

C. DETERMINE THE IMPACT ON THE EXISTING STRUCTURE FROM THE TABLE ON S2, AND VERIFY THE ADEQUACY OF THE EXISTING STRUCTURE WITH THE STRUCTURAL ENGINEER OF RECORD FOR THE BUILDING.

SHEET LIST

S1	GENERAL NOTES	S8	SHIELD WALL SYSTEM PARTS
S2	TYPICAL PARTITION WALL SECTION & BRACING SCHEDULE	S9	OPD-0001-13 DETAILS (ST6.04, ST6.05)
S3	BRACING LAYOUT PLANS	S10	OPD-0001-13 DETAILS (ST6.06, ST6.07)
S4	TOP & BOTTOM CONNECTIONS	S11	OPD-0001-13 DETAILS (ST6.08 & ST6.01)
S5	CONNECTIONS CONTD.		
S6	TEMPORARY (LESS THEN 30 DAYS) ATTACHMENT		
S7	SHIELD WALL ASSEMBLY		



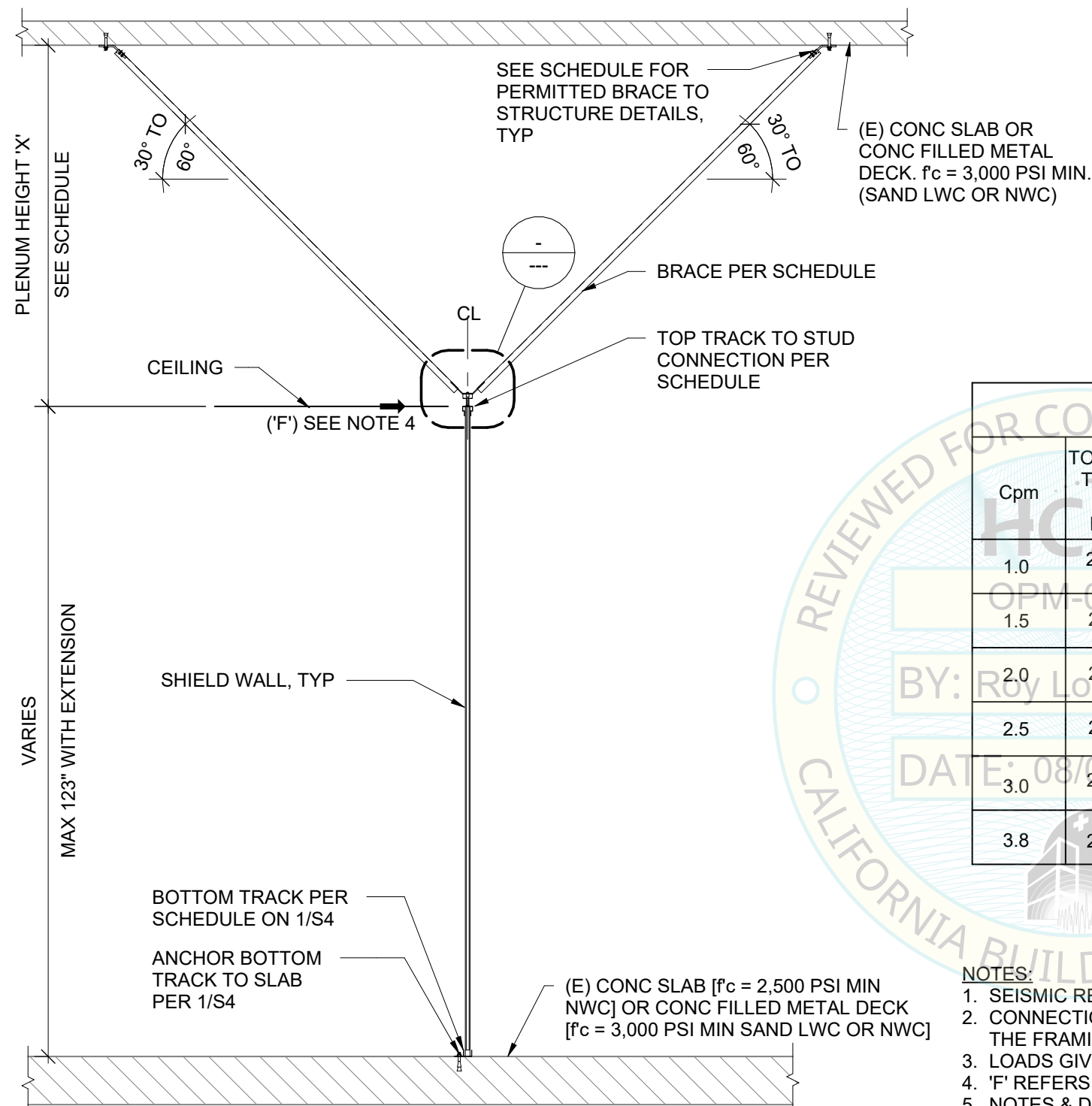
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Title
GENERAL NOTES

Drawn:	JB	Job number:	C4753066.00
Design:	JY	Rev:	
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Date	08/03/2026		

Sheet Number	S1
OF	Sheets



SCHEDULE								
Cpm	TOP TRACK TO STUD CONN DETAIL	BOTTOM TRACK CONN DETAIL	MAX KICKER SPACING	MAX ASD TOP TRACK REACTION (PLF)	MAX ASD HORIZONTAL LOAD AT BRACE (LB)	PLENUM HEIGHT	BRACE SIZE	BRACE TO STRUCTURE CONN DETAIL
1.0	2/S4, S5, 2/S6	1/S4, 1/S6	8'-0"	18	144	'X' < 5'-0"	250S162-33	S9, S10, S11
						5'-0" < 'X' < 8'-0"	(2) 362S200-33	
1.5	2/S4, S5	1/S4, 1/S6	6'-9"	26	176	'X' < 5'-0"	250S162-33	S9, S10, S11
						5'-0" < 'X' < 8'-0"	(2) 362S200-33	
2.0	2/S4, S5	1/S4, 1/S6	6'-0"	35	210	'X' < 5'-0"	250S162-33	S9, S10, S11
						5'-0" < 'X' < 8'-0"	(2) 362S200-33	
2.5	2/S4, S5	1/S4, 1/S6	5'-3"	44	231	'X' < 5'-0"	250S162-33	S9, S10, S11
						5'-0" < 'X' < 8'-0"	(2) 362S200-33	
3.0	2/S4, S5	1/S4, 1/S6	4'-9"	53	252	'X' < 5'-0"	250S162-33	S9, S10, S11
						5'-0" < 'X' < 8'-0"	(2) 362S200-33	
3.8	2/S4, S5	1/S4, 1/S6	4'-3"	67	285	'X' < 5'-0"	250S162-33	S9, S10, S11
						5'-0" < 'X' < 8'-0"	(2) 362S200-33	

- NOTES:
1. SEISMIC REACTIONS AT THE TOP AND BOTTOM CONNECTION ARE FOR A SHIELD WALL SHOWN ON S7.
 2. CONNECTION DEMANDS ARE PROVIDED TO ALLOW RDP IN RESPONSIBLE CHARGE TO VERIFY NON PRE-APPROVED COMPONENTS OF THE FRAMING SYSTEM AND THE SUPPORTING STRUCTURE.
 3. LOADS GIVEN DO NOT INCLUDE OVER-STRENGTH FACTOR (OMEGA). FOR CONCRETE ATTACHMENTS SEE ASCE 7-22 ,TABLE 13.5-1.
 4. 'F' REFERS TO THE FORCE BEING RESISTED BY CONNECTION. DEMANDS CALCULATED IN ACCORDANCE WITH ASCE 7-22.
 5. NOTES & DETAIL CALLOUTS IN SPECIFIC DETAILS TAKE PRECEDENCE OVER THE "OPD" DETAILS.
 6. MAXIMUM DESIGN WEIGHT OF WALL SYSTEM CONSIDERED IS 5 PSF.

1 TYPICAL WALL SECTION AND SCHEDULES
1/2" = 1'-0"



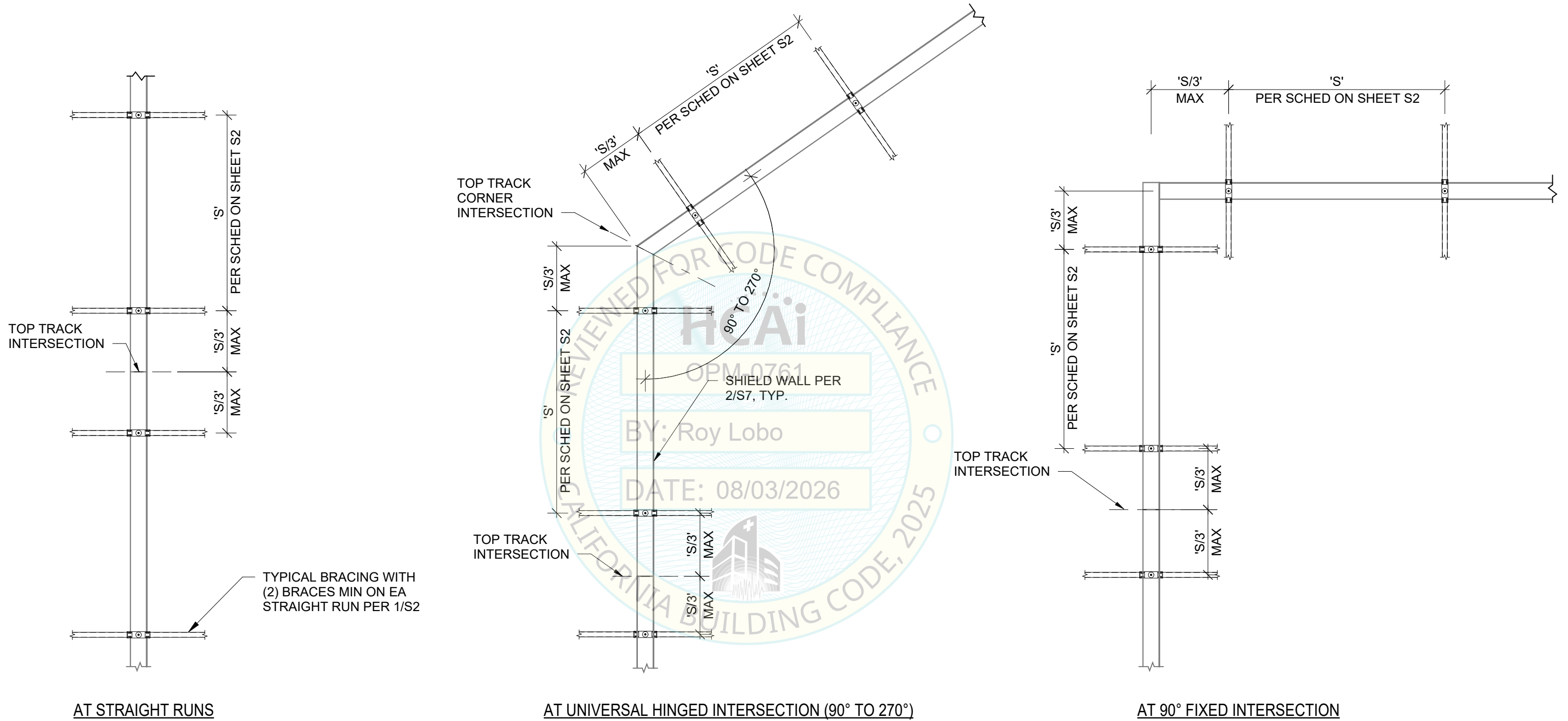
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Title
TYPICAL PARTITION WALL SECTION & BRACING
SCHEDULE

Drawn: JB Job number: C4753066.00
Design: JY Rev:
Check: CRC Scale: 1/2" = 1'-0"
Date 08/03/2026

Sheet
Number
S2
OF Sheets



1

BRACE LAYOUT PLANS

1/2" = 1'-0"



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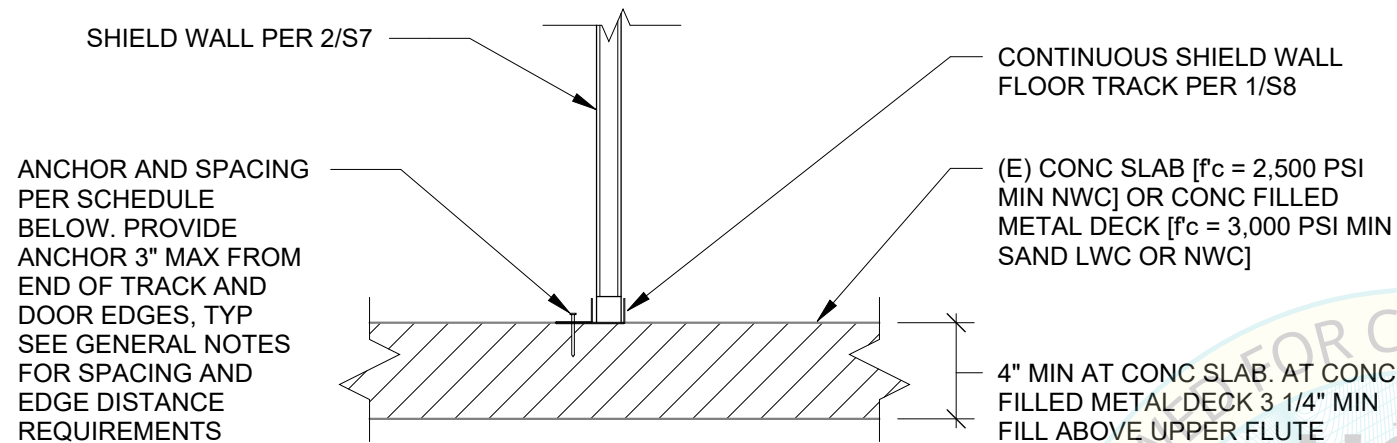


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BRACING LAYOUT PLANS

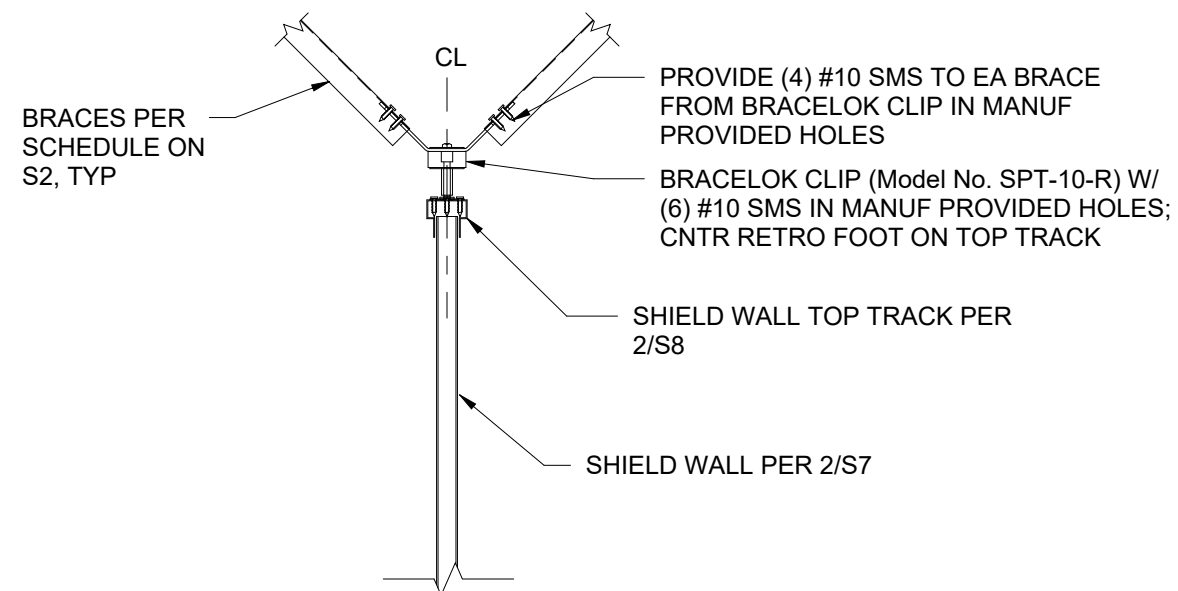
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S3

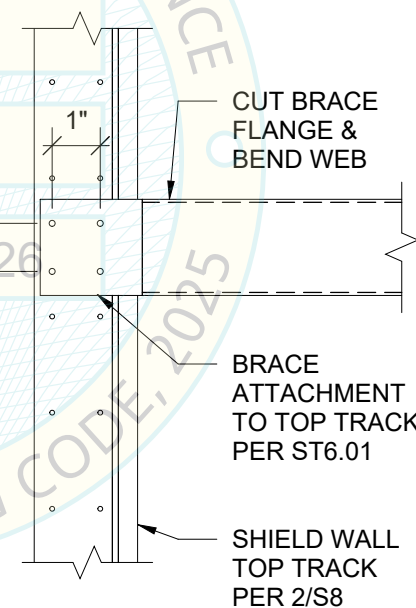
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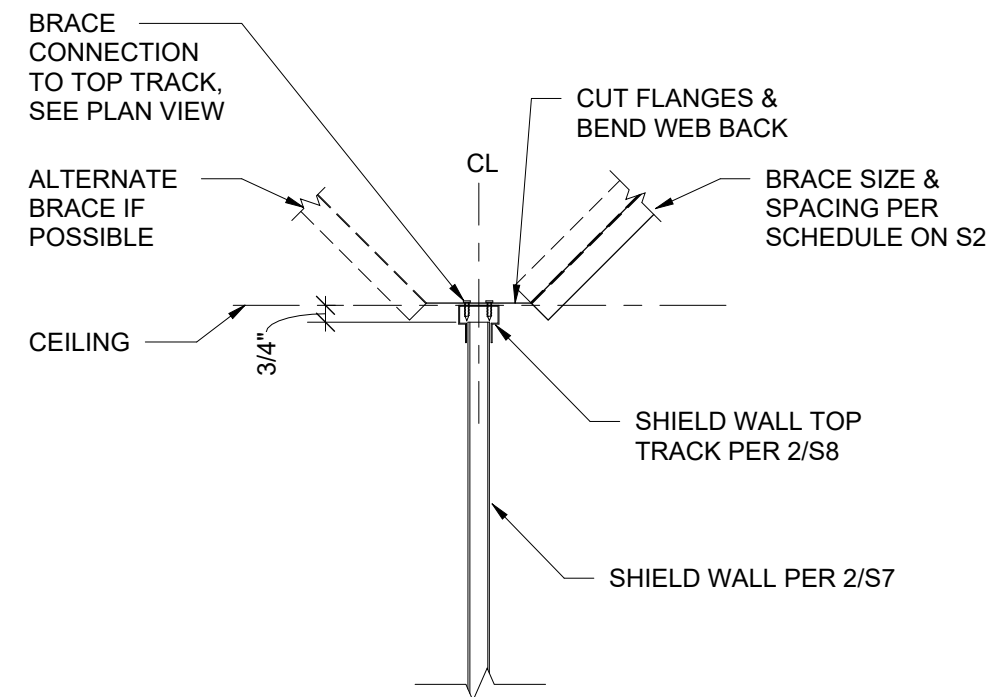
Cpm	MAX ASD BOTTOM TRACK REACTION (PLF) (DOES NOT INCLUDE Ω_0)	MAXIMUM FASTENER SPACING AT BOTTOM CONNECTION (INCHES)	
		3/8" DIA EXP ANCHOR W/ 2" EMBED (INCLUDES Ω_0)	0.157" DIA X-P PAF W/ 3/4" EMBED
1.0	18	96	62
1.5	26	96	42
2.0	35	90	31
2.5	44	81	25
3.0	53	72	21
3.8	67	66	17



BRACELOK STUD BRACING



METAL STUD BRACING - PLAN VIEW



METAL STUD BRACING - SIDE VIEW

1 BOTTOM TRACK CONNECTION

NTS

2 TOP TRACK CONNECTION

1 1/2" = 1'-0"



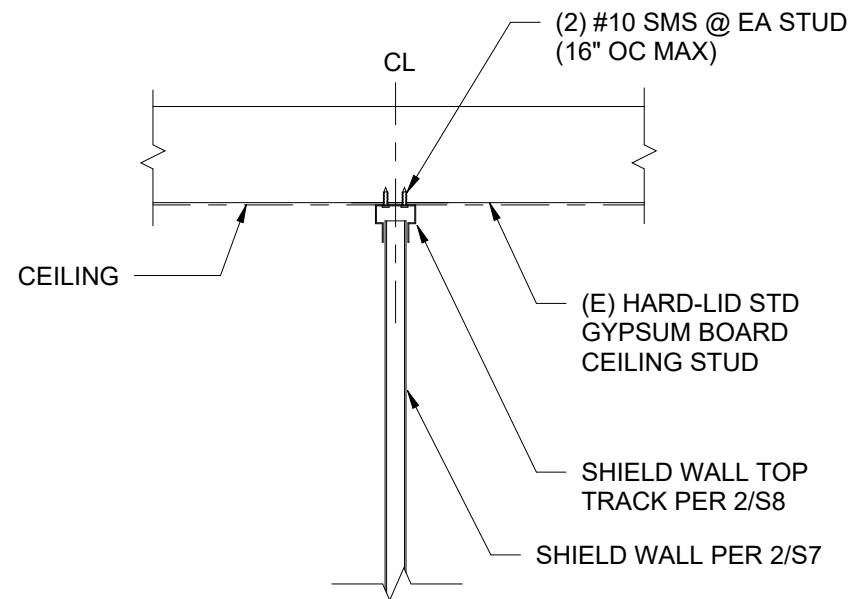
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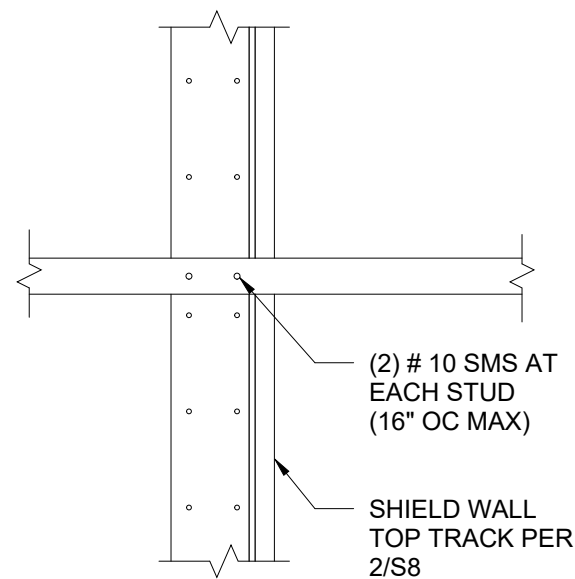
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TOP & BOTTOM CONNECTIONS

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Date 08/03/2026

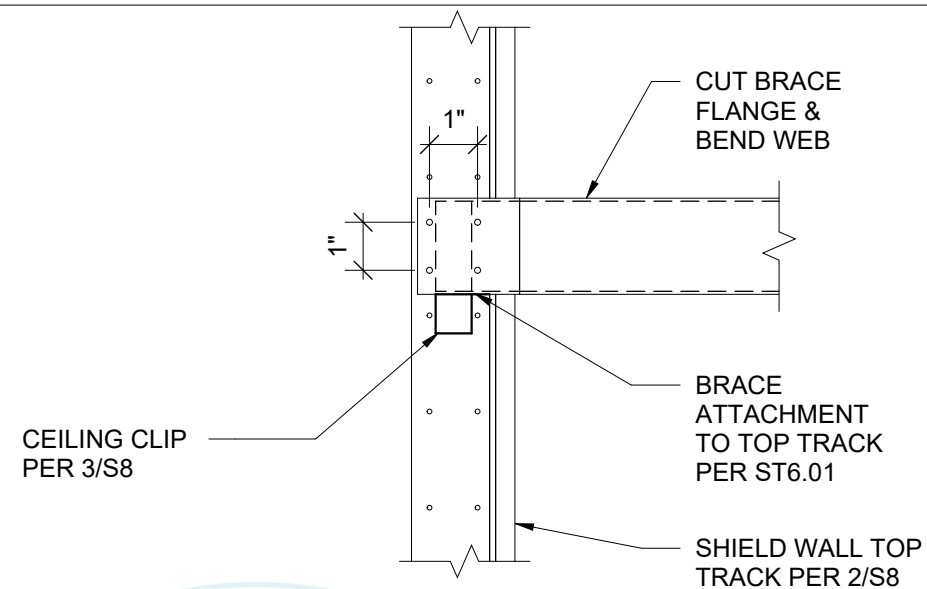
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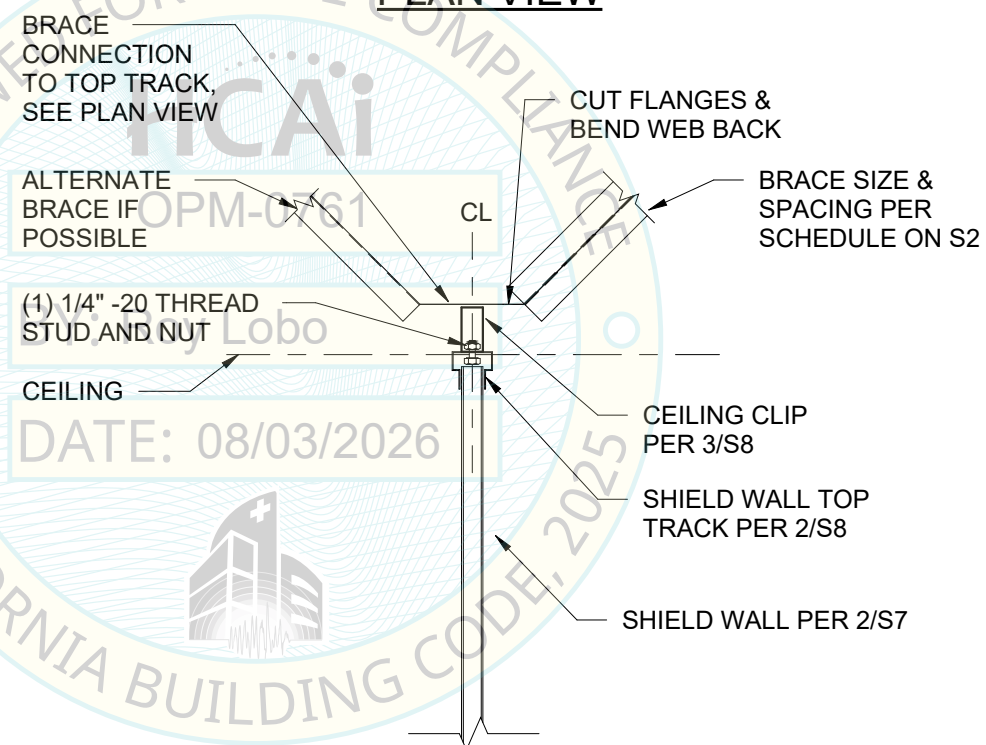
SIDE VIEW



PLAN VIEW



PLAN VIEW



SIDE VIEW

1 HARD-LID CONNECTION

1 1/2" = 1'-0"

2 CLIP CONNECTION

1 1/2" = 1'-0"



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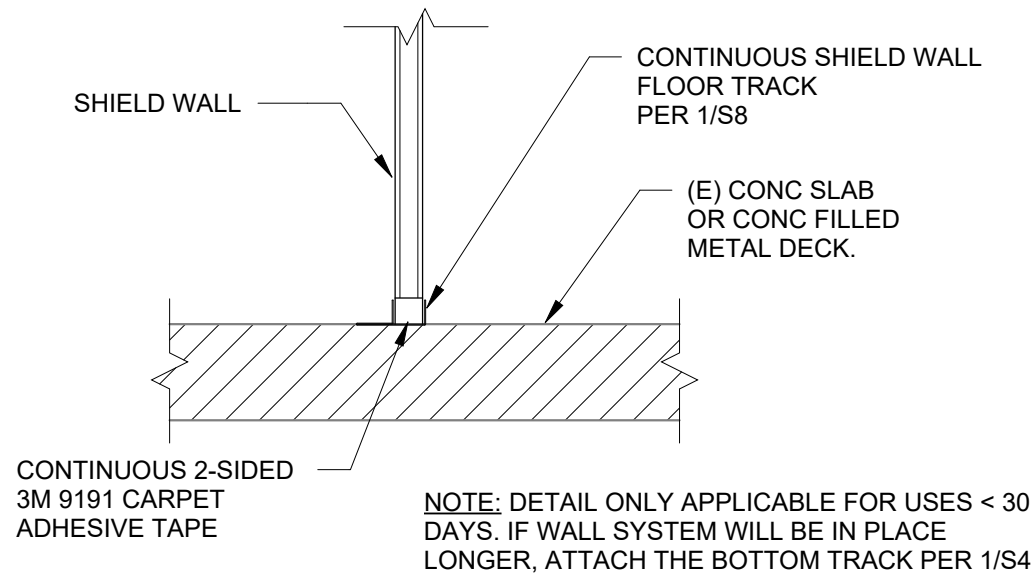


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CONNECTIONS CONTD.

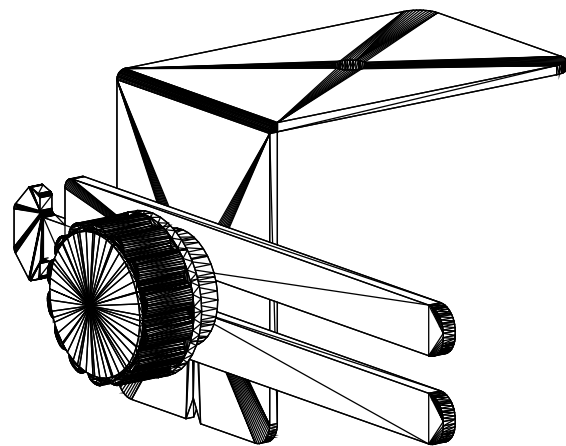
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Date:	08/03/2026		

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S5

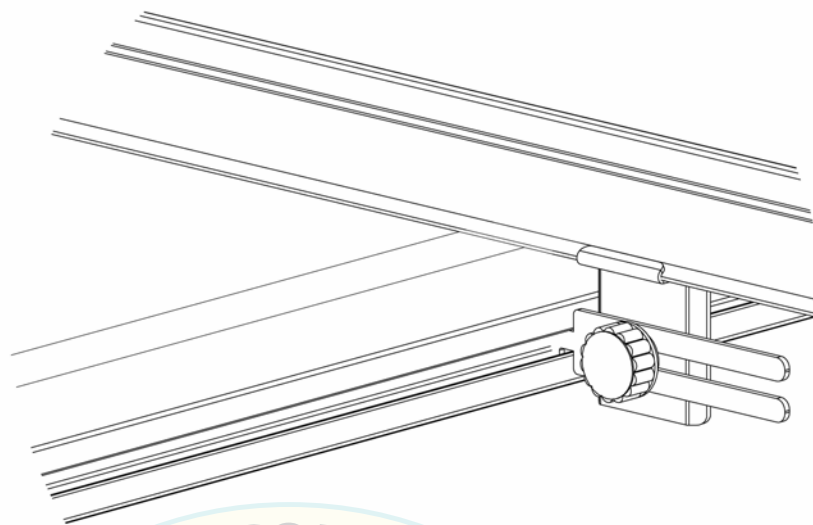
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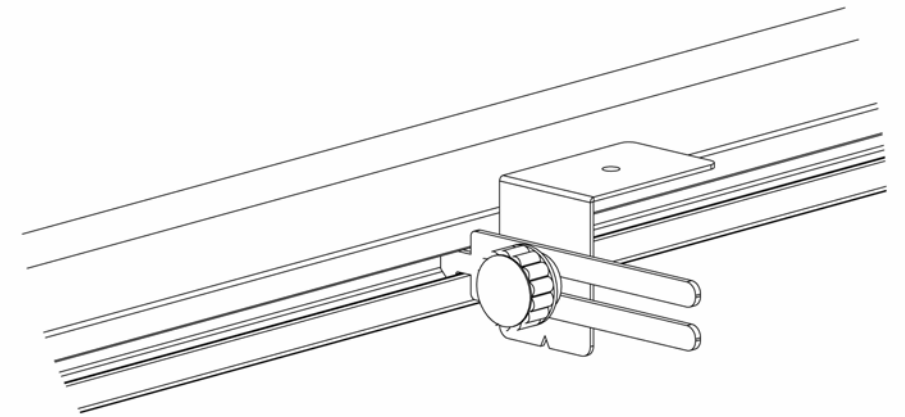
1 **BOTTOM CONNECTION**
NTS



STANDARD CEILING CLIP AT HARD-LID



STANDARD CEILING CLIP AT T-BAR (SW-HWCG)



NARROW CEILING CLIP AT T-BAR (SW-HWCGS)

T BAR CEILING

HARD LID CEILING

ISOMETRIC VIEWS (FOR REFERENCE ONLY)

NOTE:
DETAIL ONLY APPLICABLE FOR USES < 30 DAYS. IF WALL SYSTEM WILL BE IN PLACE LONGER, ATTACH THE TOP TRACK PER 2/S4 OR S5

2 **TOP CONNECTION (ERECTION CLIPS)**
NTS



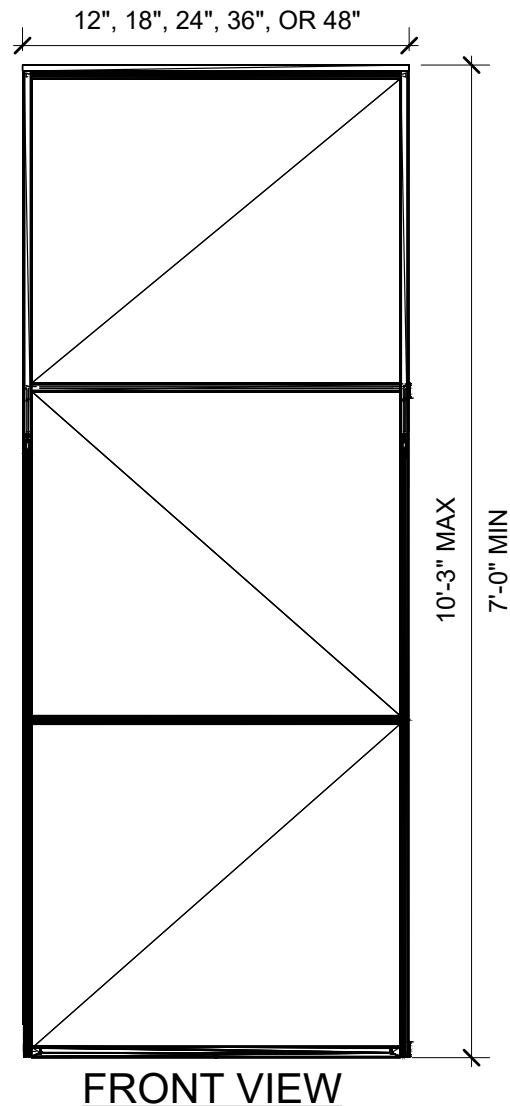
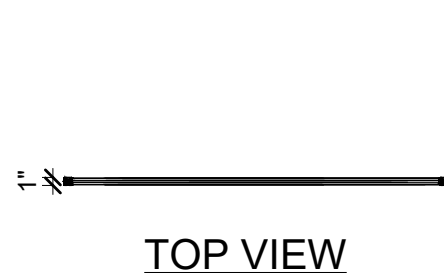
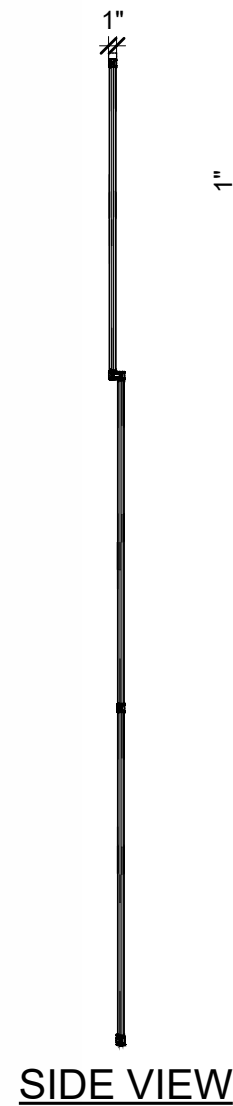
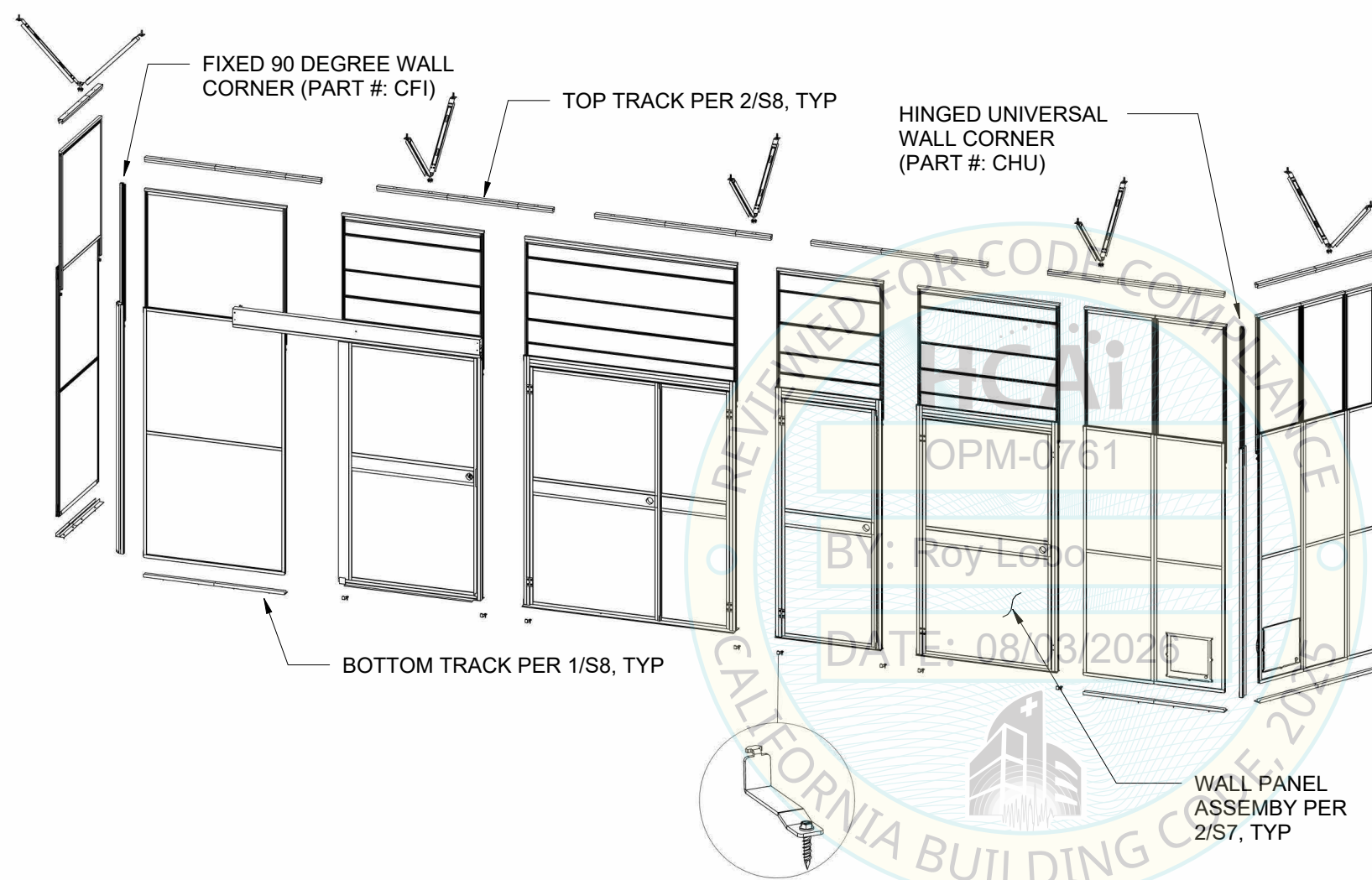
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Title
TEMPORARY (LESS THEN 30 DAYS) ATTACHMENT

Drawn:	JB	Job number:	C4753066.00
Design:	JY	Rev:	
Check:	CRC	Scale:	1 1/2" = 1'-0"
Date:	08/03/2026		

Sheet
Number
S6
OF Sheets



TYPICAL 48" WIDE PANEL
WEIGHT IS 79 LBS
(APPROX = 5 PSF FOR TOTAL
ASSEMBLY WEIGHT)

NOTE: ONLY EQUIPMENT
INTEGRAL WITH THE PROVIDED
PANELS ARE PERMITTED. NO
ADDITIONAL EQUIPMENT OR
ARCH APPENDAGES SHALL BE
ADDED TO THE PANELS PRIOR
TO OR AFTER INSTALLATION.

1 SHIELD WALL ASSEMBLY ISOMETRIC
NTS

2 TYPICAL WALL
1/2" = 1'-0"



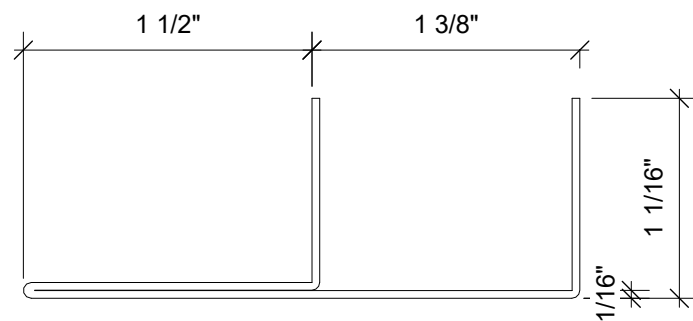
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Title
SHIELD WALL ASSEMBLY

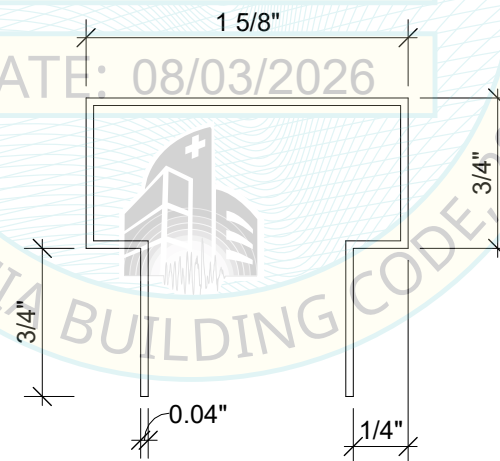
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Design:	JY	Rev:	
Check:	CRC	Scale:	As indicated
Date	08/03/2026		

Sheet
Number
S7
OF Sheets



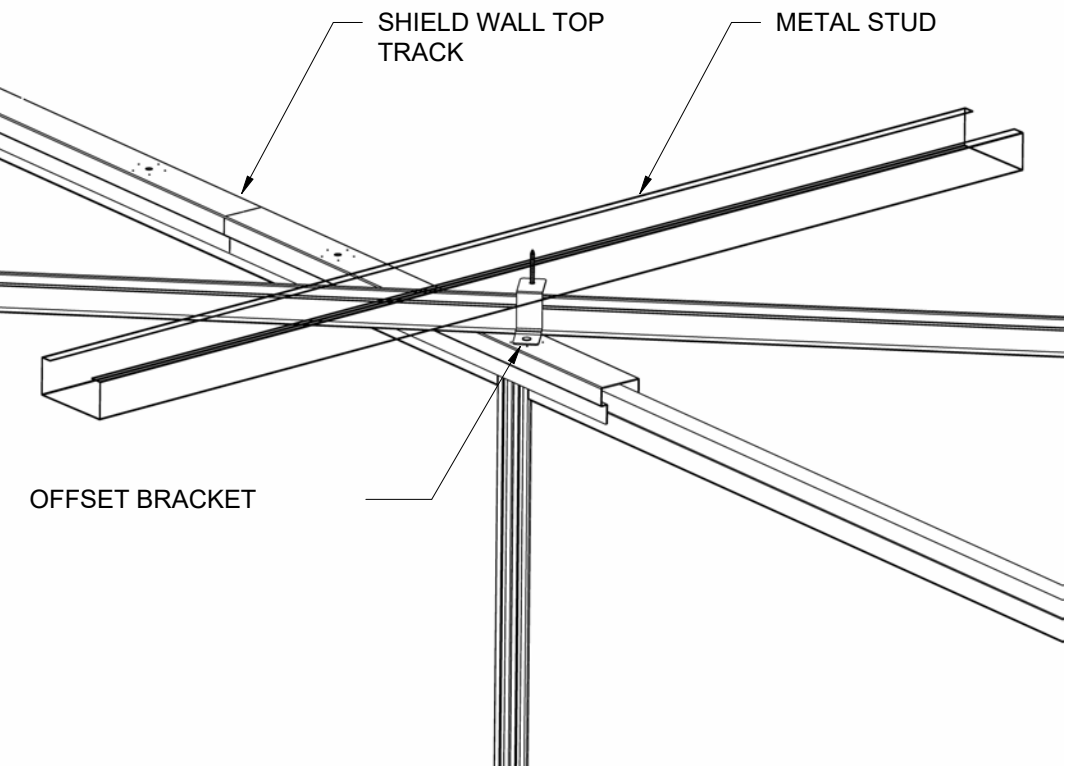
1 BOTTOM TRACK (18 GAUGE)

12" = 1'-0"

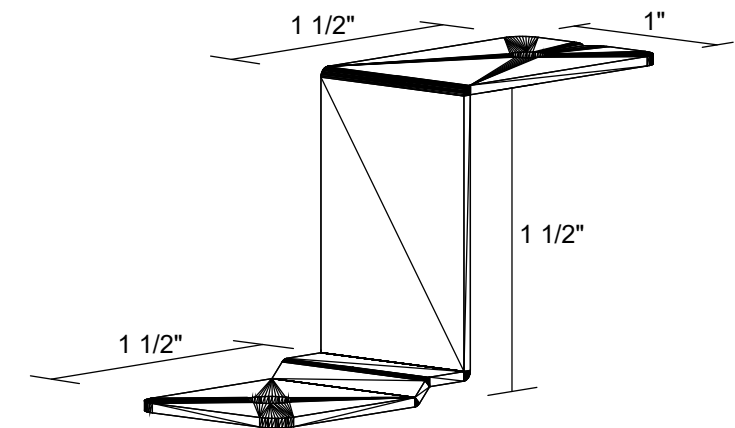


2 TOP TRACK (18 GAUGE)

12" = 1'-0"



ISOMETRIC VIEW (FOR REFERENCE ONLY)



3 CEILING CLIP OFFSET BRACKET (18 GAUGE)

NOTE: BOTTOM TRACK, TOP TRACK AND CEILING CLIP TO BE ASTM A653 (Fy = 33 KSI), MIN



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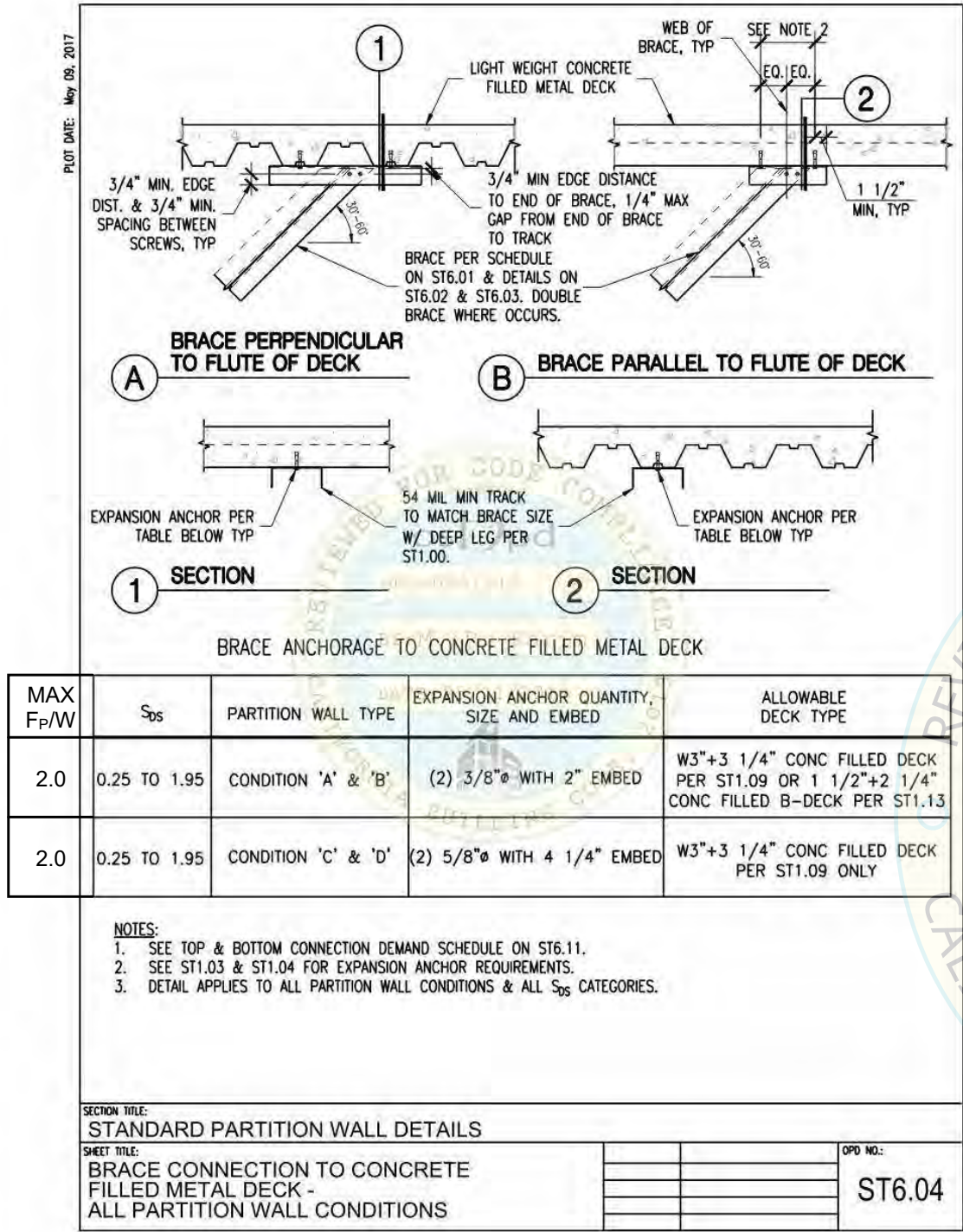


Title
SHIELD WALL SYSTEM PARTS

Drawn:	JB	Job number:	C4753066.00
Design:	JY	Rev:	
Check:	CRC	Scale:	12" = 1'-0"
Date	08/03/2026		

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Number
S8

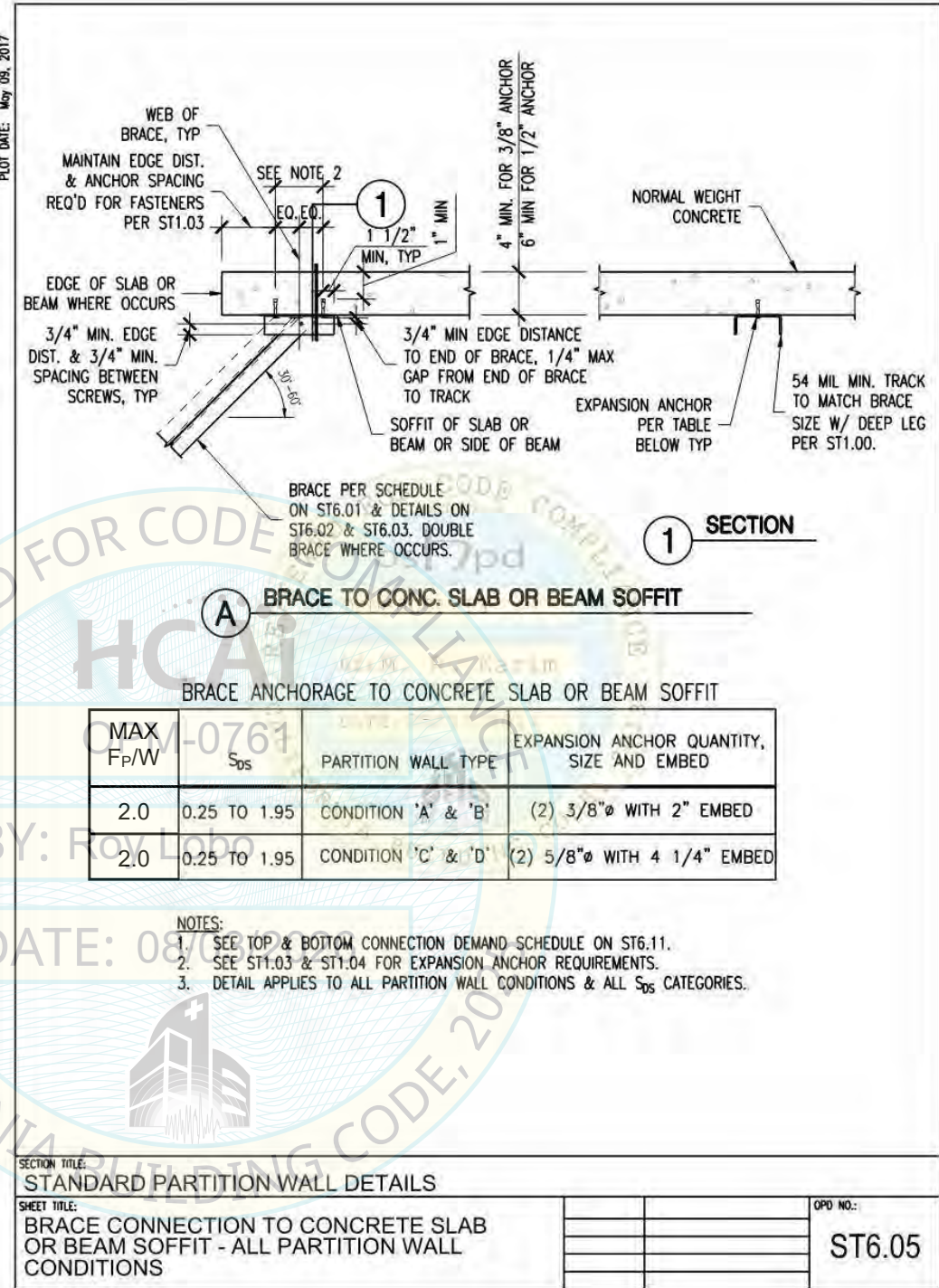
OF Sheets



1 METAL DECK ATTACHMENT



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2 CONCRETE SLAB ATTACHMENT

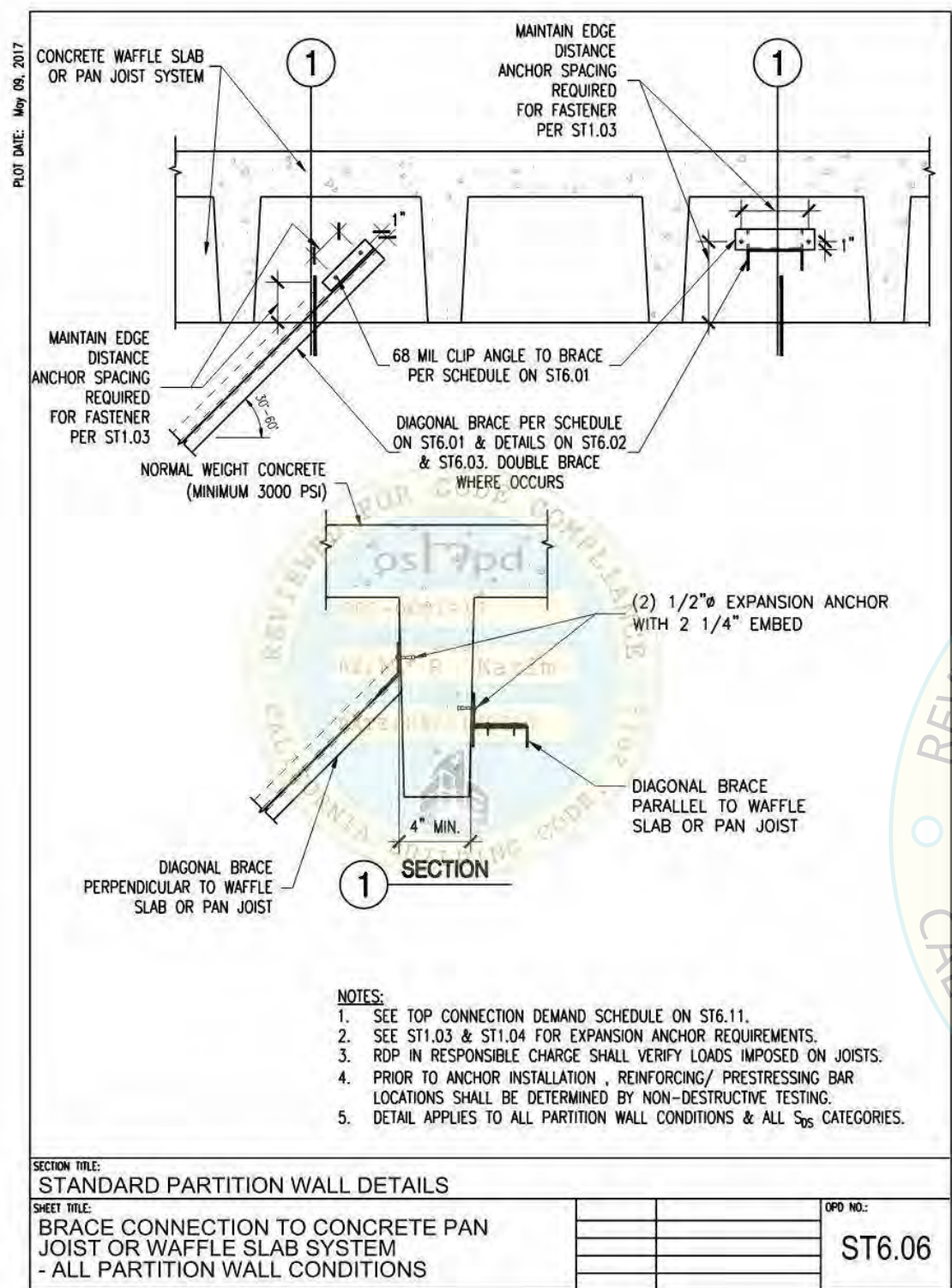


Title
OPD-0001-13 DETAILS (ST6.04, ST6.05)

- SHEET NOTES:
- THIS OPM IS BASED ON SHIELD WALL ASSEMBLY WEIGHT OF APPROX. 5 PSF.
 - SHEET NOTES TAKE PRECEDENCE OVER THE "OPD" DETAILS CALLED OUT ON THIS SHEET.
 - SEE GENERAL NOTES FOR EXPANSION ANCHOR, SHEET METAL SCREW, AND PAF REQUIREMENTS.
 - SEE SCHEDULE ON SHEET S2 FOR APPLICABLE STUD AND BRACE SIZE INFORMATION.
 - SHEETS ST6.04 AND ST6.05 USED ONLY FOR ANCHORAGE DETAILS ONLY
 - SEE SCHEDULE ST6.01 ON SHEET S11 FOR SDS VALUE AND PARTITION WALL TYPE.
 - $C_{pm} = F_P/W$

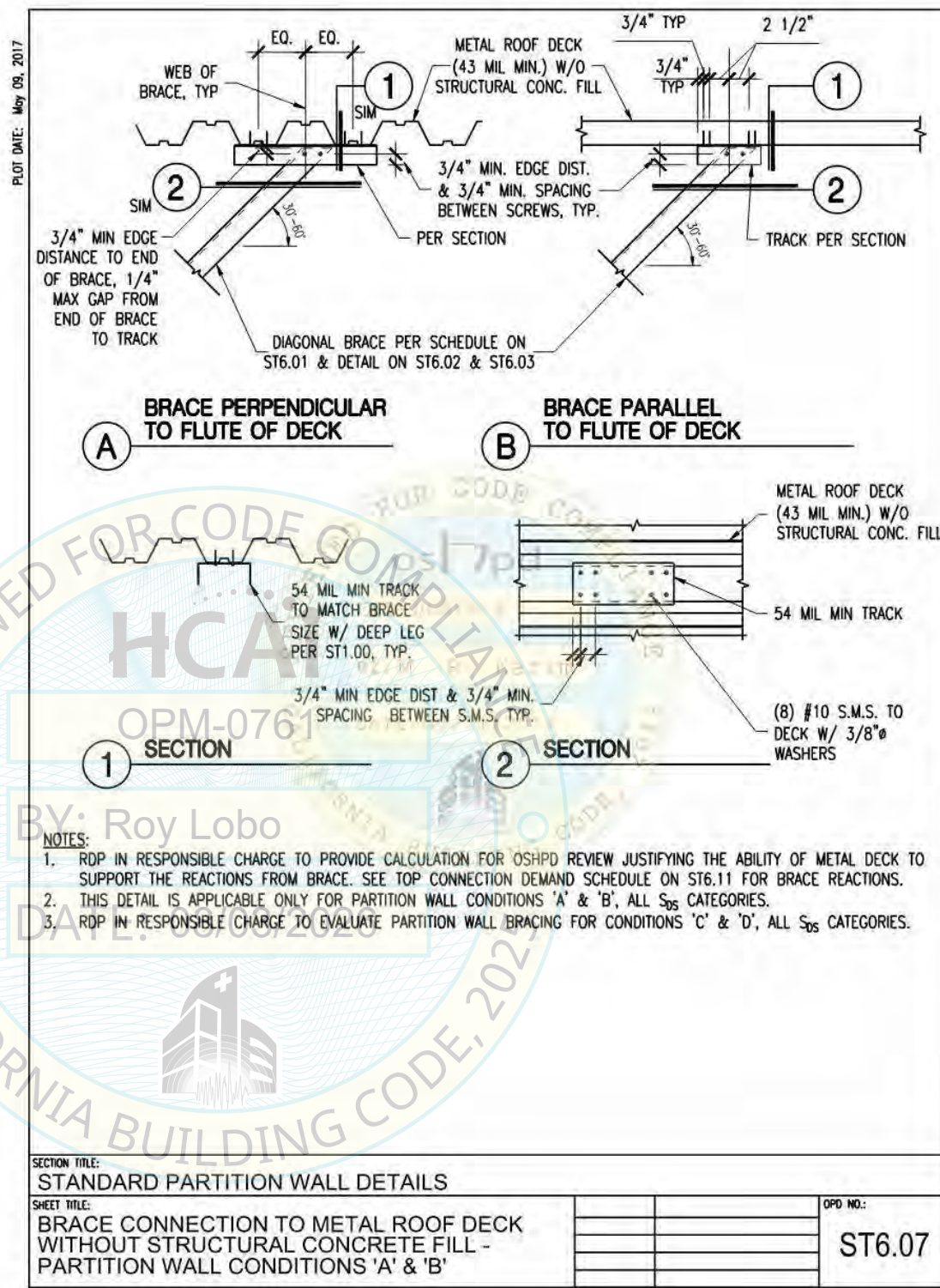
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Design: JY Rev:
Check: CRC Scale: 1" = 1'-0"
Date: 08/03/2026

Sheet Number
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OF Sheets



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1 CONCRETE JOIST ATTACHMENT



05/11/2017 OPD-0001-13; Reviewed for Code Compliance by Karim Page 60 of 86

2 BARE METAL DECK ATTACHMENT

SHEET NOTES:

1. THIS OPM IS BASED ON SHIELD WALL ASSEMBLY WEIGHT OF APPROX. 5 PSF.
2. SHEET NOTES TAKE PRECEDENCE OVER THE "OPD" DETAILS CALLED OUT ON THIS SHEET.
3. SEE GENERAL NOTES FOR EXPANSION ANCHOR, SHEET METAL SCREW, AND PAF REQUIREMENTS.
4. SEE SCHEDULE ON SHEET S2 FOR APPLICABLE STUD AND BRACE SIZE INFORMATION.
5. SHEETS ST6.06 AND ST6.07 USED ONLY FOR ANCHORAGE DETAILS ONLY
6. SEE SCHEDULE ST6.01 ON SHEET S11 FOR PARTITION WALL TYPE.



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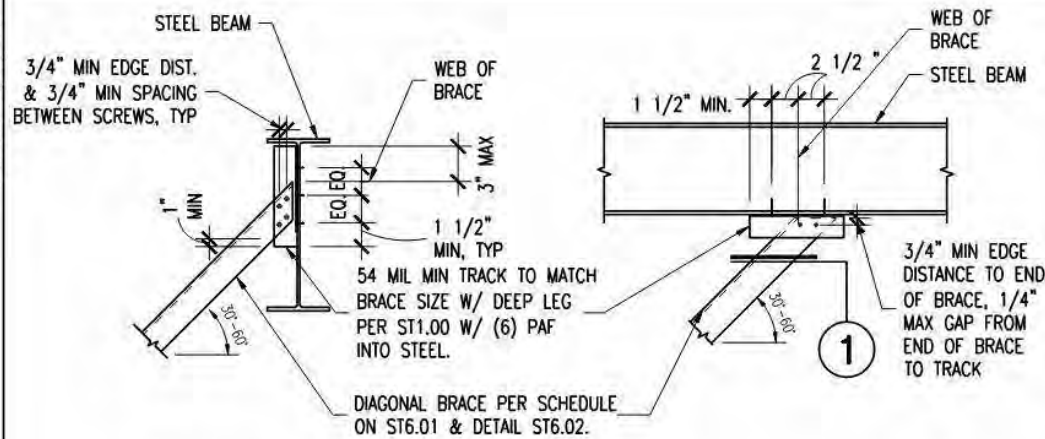


Title
OPD-0001-13 DETAILS (ST6.06, ST6.07)

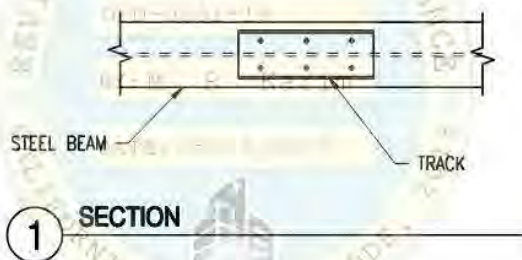
Drawn: JB Job number: C4753066.00
Design: JY Rev:
Check: CRC Scale:
Date: 08/03/2026

Sheet Number
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OF Sheets

PLOT DATE: May 09, 2017



A PERPENDICULAR TO STEEL BEAM
B BRACE PARALLEL TO STEEL BEAM



- NOTES:
- SEE TOP CONNECTION DEMAND SCHEDULE ON ST6.11.
 - SEE ST1.05 FOR PAF REQUIREMENTS.
 - THIS DETAIL IS APPLICABLE ONLY FOR PARTITION WALL CONDITIONS 'A' & 'B', ALL S_{ps} CATEGORIES.
 - STEEL BEAM FLANGE & WEB MINIMUM THICKNESS = 3/16".
 - RDP IN RESPONSIBLE CHARGE, IOR AND CONTRACTOR TO VERIFY THAT NO PDF IS INSTALLED IN THE PROTECTED ZONE OF ANY STEEL MEMBER, SEE ANSI/AISC 341-10.

SECTION TITLE: STANDARD PARTITION WALL DETAILS			
SHEET TITLE: BRACE CONNECTION TO STEEL BEAM - PARTITION WALL CONDITIONS 'A' & 'B'		OPD NO.:	ST6.08

- SHEET NOTES:**
- THIS OPM IS BASED ON SHIELD WALL ASSEMBLY WEIGHT OF APPROX. 5 PSF.
 - SHEET NOTES TAKE PRECEDENCE OVER THE "OPD" DETAILS CALLED OUT ON THIS SHEET.
 - SEE GENERAL NOTES FOR EXPANSION ANCHOR, SHEET METAL SCREW, AND PAF REQUIREMENTS.
 - SEE SCHEDULE ON SHEET S2 FOR APPLICABLE STUD AND BRACE SIZE INFORMATION.
 - SHEET ST6.08 USED ONLY FOR ANCHORAGE DETAILS ONLY
 - SEE SCHEDULE ST6.01 ON SHEET S11 FOR PARTITION WALL TYPE.

ST6.01 SCHEDULE (2025 CBC VALUES)							
Cpm	OPD SDS VALUE	OPD PARTITION WALL TYPE	MAX KICKER SPACING	MAX ASD TOP TRACK REACTION (PLF)	MAX ASD HORIZONTAL LOAD AT BRACE (LB)	S.M.S. CONNECTIONS AT BRACE ENDS	
						BOTTOM OF BRACE (TO TOP TRACK)	TOP OF BRACE (AT CONNECTION TO STRUCTURE)
1.0	0.99	A	8'-0"	18	144	(4) #10 S.M.S.	(4) #10 S.M.S.
1.5	1.45	A	6'-9"	26	176	(5) #10 S.M.S.	(4) #10 S.M.S.
2.0	1.95	A	6'-0"	35	210	(5) #10 S.M.S.	(4) #10 S.M.S.
2.5	1.45	B	5'-3"	44	231	(6) #10 S.M.S. OR (5) #10 S.M.S.	(4) #10 S.M.S.
3.0	1.95	B	4'-9"	53	252	(6) #10 S.M.S.	(4) #10 S.M.S.
3.8	1.95	B	4'-3"	67	285	(6) #10 S.M.S.	(4) #10 S.M.S.

1 STEEL BEAM ATTACHMENT

2 BRACE CONNECTION SCHEDULE ST6.01



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Title
OPD-0001-13 DETAILS (ST6.08 & ST6.01)

Drawn: JB Job number: C4753066.00
Design: JY Rev:
Check: CRC Scale: 1/2" = 1'-0"
Date: 08/03/2026

Sheet Number
S11
OF Sheets